

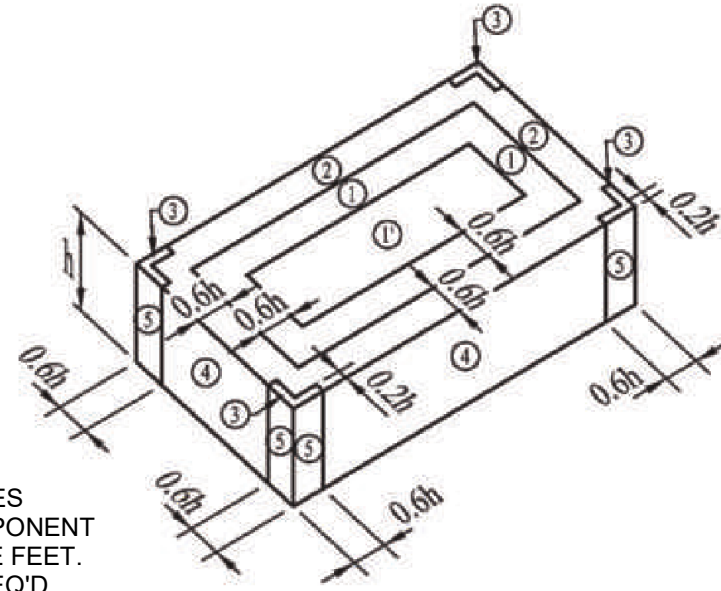
1. THE CONTRACTOR IS TO FIELD VERIFY ALL DIMENSIONS, ELEVATIONS, AND CONDITIONS PRIOR TO CONSTRUCTION OR FABRICATION.
2. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE DRAWINGS OF ALL OTHER DISCIPLINES AND ANY APPLICABLE SPECIFICATIONS. THE CONTRACTOR SHALL VERIFY THE REQUIREMENTS OF OTHER TRADES AS TO SLEEVES, HANGERS, INSERTS, ANCHORS, HOLES, AND OTHER ITEMS TO BE INCORPORATED INTO THE STRUCTURE.
3. THE STRUCTURAL DRAWINGS HEREIN REPRESENT THE FINISHED STRUCTURE. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY BRACING AND SUPPORTS REQUIRED TO ERECT AND HOLD THE STRUCTURE IN PROPER ALIGNMENT UNTIL ALL STRUCTURAL WORK, CURING, CONNECTIONS, ETC. HAVE BEEN COMPLETED. THE INVESTIGATION, DESIGN, SAFETY, ADEQUACY, AND INSPECTION OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC. IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

1. GENERAL BUILDING CODE:  
 -2018 NORTH CAROLINA STATE BUILDING CODE  
 -ACI 318-14 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE  
 -AISC STEEL CONSTRUCTION MANUAL - 14TH EDITION  
 -ASCE 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS.

2. DESIGN LOADS:  
ROOF LIVE LOAD: 20 PSF  
ROOF DEAD LOAD: 10 PSF

- |                                 |                 |
|---------------------------------|-----------------|
| 3. WIND CODE:                   |                 |
| WIND CODE:                      | ASCE 7-16       |
| BASIC WIND SPEED (Vult):        | 125 MPH         |
| OCCUPANCY CATEGORY:             | II              |
| EXPOSURE CATEGORY:              | C               |
| INTERNAL PRESSURE COEFFICIENTS: | +0.18 AND -0.18 |

4. COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TO BE USED FOR THE DESIGN OF EXTERIOR COMPONENT AND CLADDING MATERIALS NOT SPECIFICALLY DESIGNED BY THE REGISTERED DESIGN PROFESSIONAL OF RECORD.



COMPONENTS AND  
CLADDING PRESSURES  
PROVIDED FOR COMPONENT  
AREA OF 100 SQUARE FEET.  
RE-CALCULATE AS REQ'D.

ZONE	DESCRIPTION	PRESSURE (PSF)
1	INTERIOR ROOF ZONE	+16.0, -30.2
1'	INTERIOR ROOF ZONE	+16.0, -22.2
2	ROOF END ZONE	+16.0, -40.1
3	ROOF CORNER ZONE	+16.0, -47.7
4	INTERIOR WALL ZONE	+20.7, -22.7
5	WALL END ZONE	+20.7, -25.2
OVERHANG INTERIOR ZONE		-32.9
OVERHANG CORNER ZONE		-40.4

\*PRESSURES ARE ULTIMATE.

Ss:	0.206
Si:	0.086
SITE CLASS:	D
F <sub>u</sub> :	1.60
F <sub>y</sub> :	2.40
S <sub>DS</sub> :	0.220
S <sub>DI</sub> :	0.138
IMPORTANCE FACTOR:	1.0
SEISMIC USE GROUP:	II
SEISMIC DESIGN CATEGORY:	C
BASIC SEISMIC RESIST. SYSTEM:	STEEL ORDINARY MOMENT FRAMES & WOOD-FRAMED SHEAR WALLS
RESPONSE MOD. FACTOR (R):	3.5, 6.50 AT SHEAR WALLS
DESIGN BASE SHEAR (KIPS):	0.85
ANALYSIS PROCEDURE:	EQUIVALENT LATERAL FORCE

$s_g$  (GROUND SNOW LOAD): 10 PSF  
 $s_d$  (CALC. DESIGN SNOW LOAD): 9.45 PSF  
 $s_{d,min}$  (MIN. DESIGN SNOW LOAD): 10 PSF

**DESIGN CONCEPT:** BUILDING IS DESIGNED TO BE Laterally supported by wood-framed load bearing shear walls. Gravity loads are resisted by TJI joists bearing on the load bearing perimeter walls. Foundation is shallow and supported on grade.

1. STRUCTURAL FILL SHALL BE COMPACTED BETWEEN 88% AND 92% OF THE MAXIMUM DRY DENSITY AS DETERMINED BY STANDARD PROCTOR, ASTM D698, AND AT LEAST 98% OF STANDARD PROCTOR WITHIN 0'-6" BELOW CONCRETE SUBGRADE. FILL SHALL BE PLACED IN LIFTS NOT TO EXCEED 8 INCHES IN COMPACTED FILL THICKNESS.
2. FOUNDATION DESIGN IS BASED ON AN ALLOWABLE BEARING PRESSURE OF 2,000 P.S.F. AT MINIMUM 24" BELOW FINISHED SUB-GRADE LEVELS. MINIMUM CONTINUOUS FOOTING WIDTH IS 16 INCHES.
3. REMOVE ALL VEGETATION AND DEBRIS, INCLUDING PAVEMENTS, SIDEWALKS, BUILDING FOUNDATIONS, AND ABANDONED UTILITIES.
4. PROOFROLL THE EXPOSED SUBGRADE TO DETECT SOFT OR YIELDING SOILS. REMOVE ANY SOFT OR YIELDING SOILS, SCARIFY, MOISTURE CONDITION AND RECOMPACT IN ACCORDANCE WITH ASTM D-698.

5. PROVIDE A 10 MIL THICK VAPOR BARRIER BELOW THE CONCRETE SLAB. DO NOT PUNCTURE THE VAPOR BARRIER. LAP AND TAPE ENDS.
6. IT IS IMPERATIVE THAT THE ACTUAL BEARING GRADE BE CHECKED IN THE FIELD DURING CONSTRUCTION BY THE SOILS CONSULTANT TO ENSURE THAT THE FOUNDATION SYSTEM IS SUPPORTED BY SUITABLE BEARING SOILS AND WILL PERFORM IN ACCORDANCE WITH THE ASSUMED CONDITIONS.
7. PROVIDE POSITIVE DRAINAGE AWAY FROM EXCAVATIONS SO AS NOT TO ALLOW STANDING WATER FOR LONG PERIODS OF TIME.
8. SOILS EXCAVATED FROM THE SITE THAT ARE FREE OF DELETERIOUS MATERIALS MAY BE USED AS FILL.
9. FOOTING EXCAVATIONS LEFT OPEN FOR MORE THAN ONE DAY SHALL BE PROTECTED TO REDUCE EVAPORATION OR ENTRY OF SOIL MOISTURE.
10. WATER SHALL NOT BE ALLOWED TO COLLECT NEAR THE FOUNDATIONS OR FLOOR SLAB AREAS OF THE BUILDING EITHER DURING OR AFTER CONSTRUCTION. UNDERCUT OR EXCAVATION AREAS SHALL BE SLOPED TO ONE CORNER TO FACILITATE REMOVAL OF ANY COLLECTED GROUND WATER OR SURFACE RUNOFF.
11. REFER TO GEOTECHNICAL REPORT ISSUED BY DELTA OAKS GROUP, DATED 05/01/2023, FOR ALL SUBSURFACE PREPARATION. CONTRACTOR TO NOTE THAT THE SITE PREPARATION PROCEDURE CONTAINED IN THE REPORT SUPERCEDES ALL SOIL AND SITE INFORMATION PROVIDED BY ENGINEER. THE GEOTECHNICAL RECORDS OF RECORD SHALL BE RETAINED DURING CONSTRUCTION BY THE CONTRACTOR TO INSPECT EXCAVATION, PLACEMENT OF FILL, AND COMPACTION AS REQUIRED.

1. ALL CONCRETE NOT SPECIFICALLY NOTED = 4000 PSI @ 28 DAYS

2. REBAR: ASTM A615, GRADE 60  
DEFORMED WELDED WIRE FABRIC: ASTM A185
3. JOINT FILLER: VINYLFORM GRADE #300 BY SONNEBORN; CERAMAR FLEXIBLE FOAM BY W.R. MEADOWS; PLASTAZOTE BY E-POXY INDUSTRIES, INC. OR APPROVED EQUIVALENT.
4. MOISTEN SUBGRADE PRIOR TO PLACING CONCRETE.
5. FOR SPECIAL WEATHER CONCRETING - HOT & COLD WEATHER CONCRETING - FOLLOW ALL APPLICABLE ACI SPECIFICATIONS
6. CONCRETE FINISHES:
  - A. BROOM FINISH AT EXTERIOR CONCRETE:
    1. WHEN CONCRETE HAS HARDENED SUFFICIENTLY, IT SHALL BE FLOATED TO A COMPACT AND SMOOTH SURFACE. FLOATING MAY BE ACCOMPLISHED WITH WOOD OR METAL FLOATS BY MECHANICAL MEANS.
    2. AFTER FLOATING IS COMPLETED AND THE CONCRETE HAS HARDENED ENOUGH SO THAT IT WILL RETAIN THE SCORING, BRUSH THE SURFACE WITH A STEEL WIRE BROOM OR ONE MADE FROM STIFF COARSE FIBERS IN A DIRECTION TRANSVERSE TO THE DIRECTION OF TRAFFIC.
  - B. TROWELLED FINISH AT INTERIOR CONCRETE.
    1. AFTER FLOATING IS COMPLETED AND THE CONCRETE HAS HARDENED ENOUGH SO THAT WATER AND FINE MATERIAL ARE NOT WORKED TO THE SURFACE, THE SURFACE SHALL BE THOROUGHLY TROWELLED BY MACHINE TROWELLING WITH A MOTOR DRIVEN ROTARY TROWEL. THE MACHINE COMPACTING, HAND AND MACHINE TROWELLING OPERATIONS SHALL LEAVE A SMOOTH, HARD, IMPERVIOUS, EVEN FINISH.
  - C. THE HORIZONTAL SURFACES OF PUMP PADS, COLUMN PEDESTALS, TOPS OF WALLS, EQUIPMENT FOUNDATIONS REQUIRING GROUT, STAIR TREADS AND CONCRETE PADS FOR STAIRS SHALL RECEIVE A WOOD FLOAT FINISH UNLESS OTHERWISE SPECIFIED.
  - D. VERTICAL SURFACES OF EXPOSED AREAS ARE TO RECEIVE A RUBBED FINISH USING A WOOD OR STEEL FLOAT OR BURLAP TO APPLY THE GROUT. EXCESS GROUT SHALL BE REMOVED.
7. DUSTING WITH ANY MATERIAL TO ABSORB SURFACE WATER IS PROHIBITED.
8. IMMEDIATELY AFTER FORMS ARE REMOVED, FILL ALL HONEYCOMB DEPRESSIONS OR OTHER VOIDS TO OBTAIN STRAIGHT AND FLUSH SURFACES.
9. VERTICAL SURFACES SHALL HAVE ALL TIE HOLES AND SURFACE IMPERFECTIONS CORRECTED WITH MORTAR.
10. CURING:
  - A. APPLY LIQUID CURING COMPOUND TO HORIZONTAL SURFACES AS SOON AS POSSIBLE AFTER FINISHING, IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. CURE VERTICAL SURFACES BY LEAVING FORMS IN PLACE A MINIMUM OF (7) DAYS.
11. REBAR DETAILING SHALL BE PER THE LATEST EDITION OF THE ACI BUILDING CODE AND DETAILING MANUAL.

12. CONSTRUCTION JOINTS:
  - A. IN GENERAL, CONSTRUCTION JOINTS SHALL BE PLACED WHERE SHOWN ON THE DRAWINGS. JOINTS NOT SHOWN ON THE DRAWINGS SHALL BE APPROVED BY THE PROJECT ENGINEER AND SHALL BE LOCATED AND MADE SO AS TO LEAST IMPAIR THE STRENGTH OF THE STRUCTURE. ALL JOINTS SHALL BE KEVED AND DOWELED UNLESS OTHERWISE SHOWN ON THE DRAWINGS. WHERE A JOINT IS TO BE MADE, THE SURFACE OF THE CONCRETE SHALL BE THOROUGHLY CLEANED AND ALL LAITANCE REMOVED. IN ADDITION, VERTICAL JOINTS SHALL BE THOROUGHLY WETTED, AND SLUSHED WITH A COAT OF NEAT CEMENT GROUT IMMEDIATELY BEFORE PLACING OF NEW CONCRETE.
13. EARLY ENTRY SAW CUTTING SHOULD BE PERFORMED IN GREEN CONCRETE AS SOON AS SUFFICIENT CURING HAS TAKEN PLACE TO SUPPORT THE WEIGHT OF THE OPERATOR AND SAW. CUTS SHOULD BE PERFORMED USING A BLADE SPECIFICALLY DESIGNED FOR DRY EARLY ENTRY CUTTING.
14. CONTINUOUS FOUNDATION BARS SHALL HAVE SPLICES STAGGERED A MINIMUM OF 4'-0". ALL LAP SPLICES SHALL BE MADE WITH A MINIMUM LENGTH OF 36 BAR DIAMETERS. MAKE ALL BARS CONTINUOUS AROUND CORNERS BY USE OF CORNER BARS.
15. DETAIL BARS IN ACCORDANCE WITH A.C.I. DETAILING MANUAL AND A.C.I. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, LATEST EDITIONS, NO WELDING OF REINFORCEMENT SHALL BE ALLOWED UNLESS NOTED OR OTHERWISE APPROVED BY THE ARCHITECT AND STRUCTURAL ENGINEER.
16. PROVIDE THE FOLLOWING CONCRETE COVER FOR ALL REINFORCEMENT:
  - A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH = 3"
  - B. ALL OTHER AREAS = 1 1/2"

17. PROVIDE 3/4" CHAMFER OR RADIUS ON ALL EXPOSED CORNERS.
18. CONTRACTOR SHALL NOT PLACE ANY CONCRETE REINFORCEMENT UNTIL REINFORCING SHOP DRAWINGS HAVE BEEN SUBMITTED, REVIEWED AND RETURNED BY THE ARCHITECT AND STRUCTURAL ENGINEER. REINFORCING SHOP DRAWINGS SHALL CONSIST OF BOTH "CUT" AND PLACING SHEETS. PLACING SHEETS SHALL CONTAIN ALL INFORMATION NECESSARY TO POSITION ALL REINFORCING STEEL IN THE FIELD WITHOUT HAVING TO REFER TO THE STRUCTURAL DRAWINGS. ARCHITECTURAL AND STRUCTURAL DRAWINGS ARE NOT PERMITTED FOR USE AS SHOP DRAWINGS. ARCHITECTURAL AND STRUCTURAL DRAWINGS SHALL NOT BE REPRODUCED IN ANY MANNER FOR USE AS SHOP DRAWINGS.
19. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN, ENGINEERING, STRUCTURAL ADEQUACY, AND CONSTRUCTION OF ALL SOIL RETAINING STRUCTURES AND CONCRETE SHORING AND FORMWORK IN ACCORDANCE WITH THE CONTRACT SPECIFICATIONS.
20. COORDINATE ALL CONCRETE WORK WITH THE PLACEMENT OF PIPING, INSERTS, FLOOR DRAINS, AND OTHER EMBEDDED ITEMS INDICATED ON THE CONTRACT DRAWINGS.

1. WOOD FRAMING SHALL COMPLY WITH THE WEST COAST LUMBER INSPECTION BUREAU.
2. STRUCTURAL WOOD FRAMING SHALL BE 2 INCHES X 6 INCHES AND LARGER SHALL BE NO.2 DOUGLAS FIR OR EQUIVALENT (U.N.O.).
3. WOOD COLUMNS SHALL BE 6 INCHES X 6 INCHES AND LARGER SHALL BE NO.1 SOUTHERN PINE OR EQUIVALENT.
4. ALL EXPOSED WOOD RAFTERS AND COLUMNS SHALL BE "SELECT" GRADE AS DESCRIBED IN AITC.
5. ALL PLATES IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED FOR MOISTURE PROTECTION.
6. REFER TO MIN. FASTENING SCHEDULE, IBC TABLE 2304.10.1 FOR MINIMUM FASTENING REQUIREMENTS FOR ALL WOOD FRAMING U.N.O. ON CONSTRUCTION DOCUMENTS
7. STUD PACKS
  - A. EXTERIOR CORNERS SHALL BE MADE WITH SOLID FULL HEIGHT 2x6 STUDS AND ASSEMBLED WITH MINIMUM 10D NAILS AT 8" ON CENTER. WHERE CORNER HOLD-DOWNS ARE ATTACHED TO STUD PACKS AT CORNERS, DOORS, WINDOWS AND ELSEWHERE, THE STUDS SHALL BE ASSEMBLED WITH A DOUBLE ROW OF 10D NAILS AT 8" ON CENTER.
  - B. EXTERIOR WALL TEES SHALL BE MADE WITH AT LEAST THREE (3) FULL HEIGHT STUDS ASSEMBLED WITH 10D NAILS AT 8" ON CENTER.
  - C. EXTERIOR WALL TEES SHALL BE MADE WITH AT LEAST TWO (2) FULL HEIGHT STUDS WITH FLATWISE BLOCKING WITH A SUM TOTAL LENGTH OF AT LEAST 50% OF THE WALL HEIGHT ASSEMBLED WITH 10D NAILS AT 8" ON CENTER.

1. ALL ROOF DECK SHALL BE APA RATED STRUCTURAL I EXTERIOR PLYWOOD
2. ROOF SHEATHING SHALL BE 5/8 INCH THICK MINIMUM U.O. STAGGER ROOF PANELS AND PROVIDE USP CLIPS AT EACH PANEL JOINT.
3. ALL ROOF FASTENERS SHALL BE MINIMUM 8D (0.133 X 2-3/8") IRREGULAR SHANK (I.E., RING SHANK OR SPIRAL) NAILS WITH FULL ROUND HEADS. STAPLES ARE NOT PERMITTED FOR FASTENING OF THE ROOF DECKING.
4. ROOF DECKS SHALL BE NAILED IN ACCORDANCE WITH THE FOLLOWING:
  - 6 INCHES ON CENTER ALONG PANEL EDGES AND 12" IN THE FIELD FOR ZONE 1
  - 4 INCHES ON CENTER ALONG PANEL EDGES AND 12" IN THE FIELD FOR ZONES 2 & 3
  - REFER TO WIND DIAGRAM ON THIS SHEET FOR DESCRIPTION OF ZONES

1. ALL FOUNDATION ANCHOR BOLTS TO BE F1554 GRADE 36 STEEL UNLESS SPECIFICALLY NOTED OTHERWISE
2. LOCATE ANCHOR BOLTS ACCURATELY, SET WITH TEMPLATES, AND SHOW POSITION HOLDS IN PLACING CONCRETE. PROTECT IN-PLACE ANCHOR BOLTS FROM CONSTRUCTION ACTIVITY.
3. INSERTING ANCHOR BOLTS INTO FRESH OR PARTIALLY HARDENED CONCRETE IS PROHIBITED. SUBSTITUTING CONCRETE/EXPANSION OR ADHESIVE ANCHORS WHERE EMBEDDED ANCHOR BOLTS ARE SHOWN IS PROHIBITED UNLESS NOTED OTHERWISE.
4. ALL ANCHOR BOLTS SHALL BE DELIVERED TO THE JOB SITE FREE OF EXCESS RUST, MILL SCALE, ETC.

1. STRUCTURAL STEEL CONSTRUCTION SHALL MEET THE LATEST PROVISIONS OF THE AISC SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS.
2. STEEL W-SHAPES SHALL CONFORM TO ASTM SPEC. A992
3. STEEL PLATES AND BAR SHALL CONFORM TO ASTM SPECIFICATION ASTM A36
4. STEEL ANGLES AND CHANNELS SHALL CONFORM TO ASTM SPECIFICATION ASTM A36
5. STEEL TUBE SECTIONS SHALL CONFORM TO ASTM A500, GRADE B.
6. STRUCTURAL STEEL SHALL BE DETAILED AND FABRICATED IN ACCORDANCE WITH THE LATEST PROVISIONS OF THE 13th EDITION OF THE A.I.S.C. MANUAL OF STEEL CONSTRUCTION.
7. OPENINGS SHALL NOT BE ALLOWED IN STRUCTURAL STEEL MEMBERS UNLESS SPECIFICALLY DETAILED ON THE DRAWINGS OR WITHOUT APPROVAL FROM THE STRUCTURAL ENGINEER.
8. ALL STRUCTURAL STEEL SHALL BE DELIVERED TO THE JOB SITE FREE OF EXCESS RUST, MILL SCALE, GREASE, ETC.
9. BEARING (N) TYPE CONNECTIONS SHALL BE USED AT ALL SIMPLE SHEAR CONNECTIONS, UNLESS NOTED OR SHOWN OTHERWISE.
10. THE STRUCTURAL STEEL FABRICATOR SHALL FURNISH SHOP AND ERECTION DRAWINGS FOR ALL STRUCTURAL STEEL FOR ARCHITECTS' AND STRUCTURAL ENGINEERS' REVIEW BEFORE FABRICATION. CONTRACTOR SHALL NOT ERECT ANY STRUCTURAL STEEL UNTIL THE SUBMITTED DRAWINGS ARE REVIEWED AND RETURNED BY THE ARCHITECT AND STRUCTURAL ENGINEER.

1. THE CONTRACTOR SHALL INCLUDE IN THE BID THE COST OF ALL TESTING AND INSPECTIONS INDICATED ON THE PLANS AND IN THE SPECIFICATIONS, INCLUDING SPECIAL INSPECTIONS REQUIRED BY THE BUILDING CODE. THE ACTUAL CONTRACTING OF THE INSPECTION AND TESTING SERVICES SHALL BE IN ACCORDANCE WITH THE DIVISION OF RESPONSIBILITY DICTATED BY THE LOCAL BUILDING CODE.
2. IBC SECTION 1704 REQUIRES THAT IN ADDITION TO THE INSPECTIONS REQUIRED BY SECTION 109, THE OWNER SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS WHO SHALL PROVIDE INSPECTIONS DURING CONSTRUCTION OF CERTAIN TYPES OF WORK.
3. "SPECIAL INSPECTION" CONCERNS WORK REQUIRING OBSERVATION AND JUDGEMENT AND SHALL BE PERFORMED BY A STRUCTURAL ENGINEER (OR A DESIGNATED PERSON UNDER THE SUPERVISION OF THE ENGINEER). "TESTING" INVOLVES THE ANALYSIS OF MATERIALS IN ACCORDANCE WITH APPROVED STANDARDS AND SHALL BE PERFORMED BY AN INDEPENDENT TESTING AGENCY.
4. THE CONTRACTOR SHALL COORDINATE THE TESTING AND INSPECTION SERVICES IN ACCORDANCE WITH THE PROGRESS OF THE WORK. THE CONTRACTOR SHALL PROVIDE SUFFICIENT PRIOR NOTICE TO THE TESTING AND/OR INSPECTION AGENCY OF THE REQUIRED WORK TO ALLOW PROPER SCHEDULING OF PERSONNEL. THE COST OF ANY RETESTING OR ADDITIONAL INSPECTIONS AS A RESULT OF FAILED TESTS AND REJECTED WORK SHALL BE BORNE BY THE CONTRACTOR.
5. THIS PROJECT REQUIRES A STRUCTURAL TESTING AND SPECIAL INSPECTION SCHEDULE, TO BE SIGNED BY THE OWNER, CONTRACTOR, ARCHITECT, STRUCTURAL ENGINEER OF RECORD AND TESTING AGENCY. REFER TO THE PROGRAM SUMMARY SCHEDULE FOR FREQUENCY OF TESTING AND INSPECTIONS.

REQUIRED SPECIAL INSPECTIONS					
DESCRIPTION OF WORK IBC SECTION 1704	INSPECTION		TESTING		REMARKS
	YES	NO	YES	NO	
CONCRETE CONSTRUCTION (1704.4)	●		●		6A
SOILS (1704.7)		●	●		6B
STEEL (1704.3)	●				6C

6. SPECIAL INSPECTION WORK REQUIRED INCLUDE CONVENTIONAL TESTING NOT STRICTLY A PART OF SECTION 1704 BUT ARE REQUIRED FOR ADEQUATE QUALITY ASSURANCE AND CAN BE PROVIDED BY THE CONTRACTOR. ALL OTHER WORK MUST BE PROVIDED BY THE OWNER AS INDICATED BY THE INTERNATIONAL BUILDING CODE
- 6A. CONCRETE TESTING
- A. PROVIDE MIX DESIGN IN ACCORDANCE WITH ACI REQUIREMENTS.
  - B. TEST CONCRETE AT THE TIME OF POURING FOR SLUMP, AIR-ENTRAINMENT, AND TEMPERATURE IN ACCORDANCE WITH THE SPECIFICATIONS
  - C. MAKE AND TEST CONCRETE CYLINDERS FOR REPRESENTATIVE STRENGTH IN ACCORDANCE WITH THE SPECIFICATIONS.
  - D. PROVIDE PERIODIC VISUAL INSPECTION OF REINFORCING:
    - VISUAL INSPECTION OF 25% OF CONTINUOUS STRIP FOOTINGS PRIOR TO POUR
    - VISUAL INSPECTION OF 50% OF SLABS ON GRADE
- 6B. SOILS
- A. VERIFY FOOTING EXCAVATION FOR SUITABILITY FOR PLANNED FOOTING.
  - B. VERIFY MATERIAL USED FOR COMPACTED BACKFILL.
  - C. TEST COMPACTED BACKFILL FOR SPECIFIED COMPACTION.
- 6C. STEEL
- A. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS
  - B. REVIEW PRE-ENGINEERED BUILDING MANUFACTURER'S CERTIFICATE OF COMPLIANCE
  - D. INSPECTION OF HIGH-STRENGTH BOLTED CONNECTION TYPES
  - E. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS
  - F. MATERIAL VERIFICATION OF WELD FILLER MATERIALS
  - G. INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE WITH APPROVED CONNECTION DOCUMENTS:
    - DETAILS SUCH AS BRACING AND STIFFENING
    - MEMBER LOCATIONS
    - APPLICATION OF DETAILS AT EACH CONNECTION

**DESIGN LOADS:**

**Importance Factors:** Snow ( $I_s$ ) 1.0  
Seismic ( $I_e$ ) 1.0

**Live Loads:** Roof \_\_\_\_\_ psf  
Mezzanine \_\_\_\_\_ N/A psf  
Floor \_\_\_\_\_ 100 psf

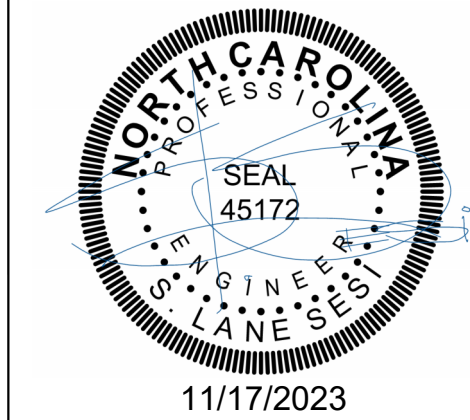
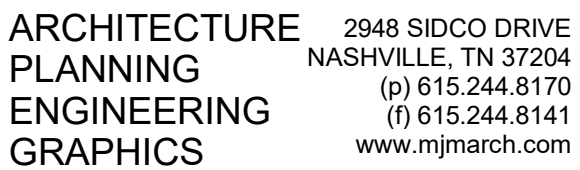
**Ground Snow Load:** \_\_\_\_\_ 10 psf

**Wind Load:** Ultimate Wind Speed \_\_\_\_\_ 125 mph (ASCE-7)  
Exposure Category B

Provide the following Seismic Design Parameters:

<b>Risk Category</b> (Table 1604.5)	II	
<b>Spectral Response Acceleration</b>	$S_s$ 0.206 %g	$S_1$ 0.086 %g
<b>Site Classification</b> (ASCE 7)	D	
Data Source:	D	<u>Presumptive</u>
<b>Basic structural system</b>	<u>Bearing Wall</u>	
<b>Analysis Procedure:</b>	<u>Equivalent Lateral Force</u>	
<b>Architectural, Mechanical, Components anchored?</b>	No	

**SOIL BEARING CAPACITIES:**  
 Field Test (provide copy of test report) \_\_\_\_\_ 2,000 \_\_\_\_\_ psi  
 Pile size, type, and capacity \_\_\_\_\_ N/A (Shallow foundations) \_\_\_\_\_



MOUNT OAK  
CAPITAL, LLC

500 6TH STREET  
SAN ANTONIO, TX 78215

SBUX SHELL

503 WEST THIRD ST.  
PEMBROKE, NC 28372

PERMIT SET

11/02/23  
23331

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## STRUCTURAL GENERAL NOTES

# S001








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## REVISIONS

**S101**



LEGEND	
	EARLY ENTRY SAW CUT CONTROL JOINT; REFER TO TYP. DETAILS
	LOAD BEARING SHEAR WALL; 2x6 AT 16" O.C. WITH 2x6 BLOCKING AT 48" O.C. MAX. REFER TO TYPICAL SHEAR WALL DETAIL, S301
	NON-LOAD BEARING KNEE WALL; 2x6 AT 16" O.C. WITH (2) 2x6 SILL PLATE AND TOP PLATE; ATTACH STUDS TO SILL PLATE AND CONCRETE WITH SIMPSON RCKW5.5 WITH (2) 1/2" DIA. ANCHORS EMBEDDED INTO 4" INTO SLAB

SPREAD FOOTING SCHEDULE					
TYPE	WIDTH	LENGTH	FOOTING THICKNESS	REINFORCING BOTTOM	REINFORCING TOP
F48	4' - 0"	4' - 0"	1' - 0"	#5 AT 10" O.C. EACH WAY	#5 AT 10" O.C. EACH WAY
F36	3' - 0"	3' - 0"	1' - 0"	#5 AT 10" O.C. EACH WAY	#5 AT 10" O.C. EACH WAY
GB1	3' - 0"	SEE PLAN	1' - 0"	#5 AT 10" O.C. EACH WAY	



500 6TH STREET  
SAN ANTONIO, TX 78215



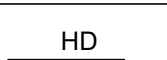
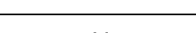
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PEMBROKE, NC 28372

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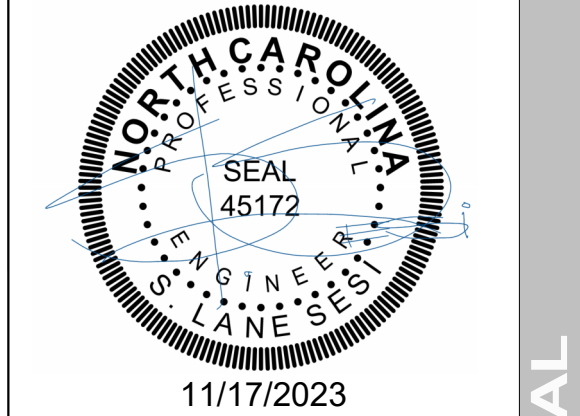
## REVISIONS

S102 SHEET



LEGEND	
	STEEL TUBE COLUMN; REFER TO S101 FOR SIZE AND LOCATION
	LOAD BEARING SHEAR WALL; 2x6 AT 16" O.C. WITH 2x6 BLOCKING AT 48" O.C. MAX. REFER TO TYPICAL SHEAR WALL DETAIL, S301
	SHEAR WALL WITH (2) HDU5-SDS2.5 SIMPSON HOLDOWNS U.N.O. WHERE SHOWN AND AT CORNER CONDITIONS; PROVIDE MINIMUM 4-PLY AND 2-PLY STUD PACK; REFER TO TYPICAL HOLDOWN DETAIL, S301
	(3)2x8 HEADER IN LOAD BEARING WALL WITH HOLDOWNS ON EACH SIDE. REFER TO DETAIL 12, SHEET S301





MOUNT OAK CAPITAL, LLC

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SBUX SHELL

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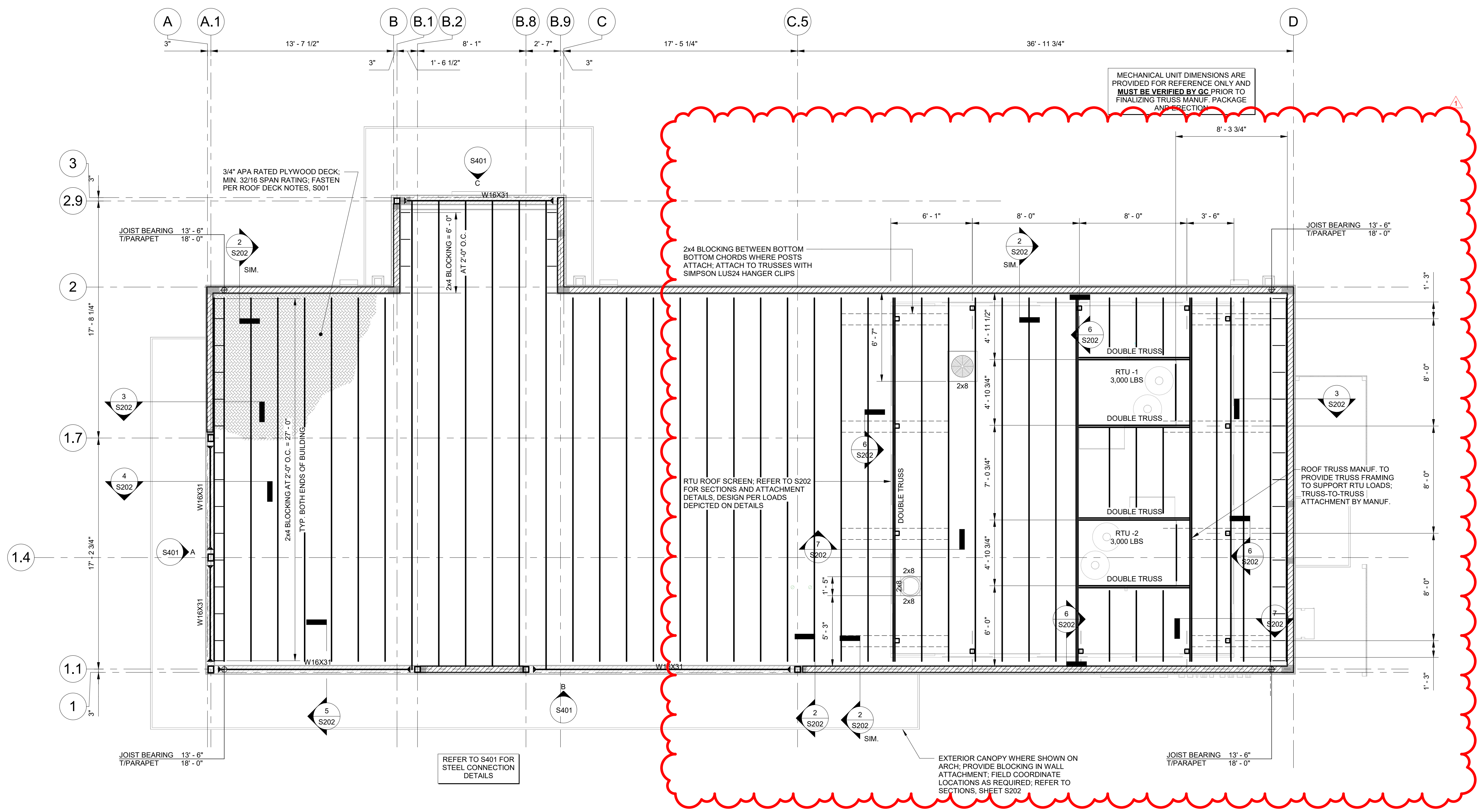
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NO.	DESCRIPTION	DATE
1	REVISION 1	11/16/23

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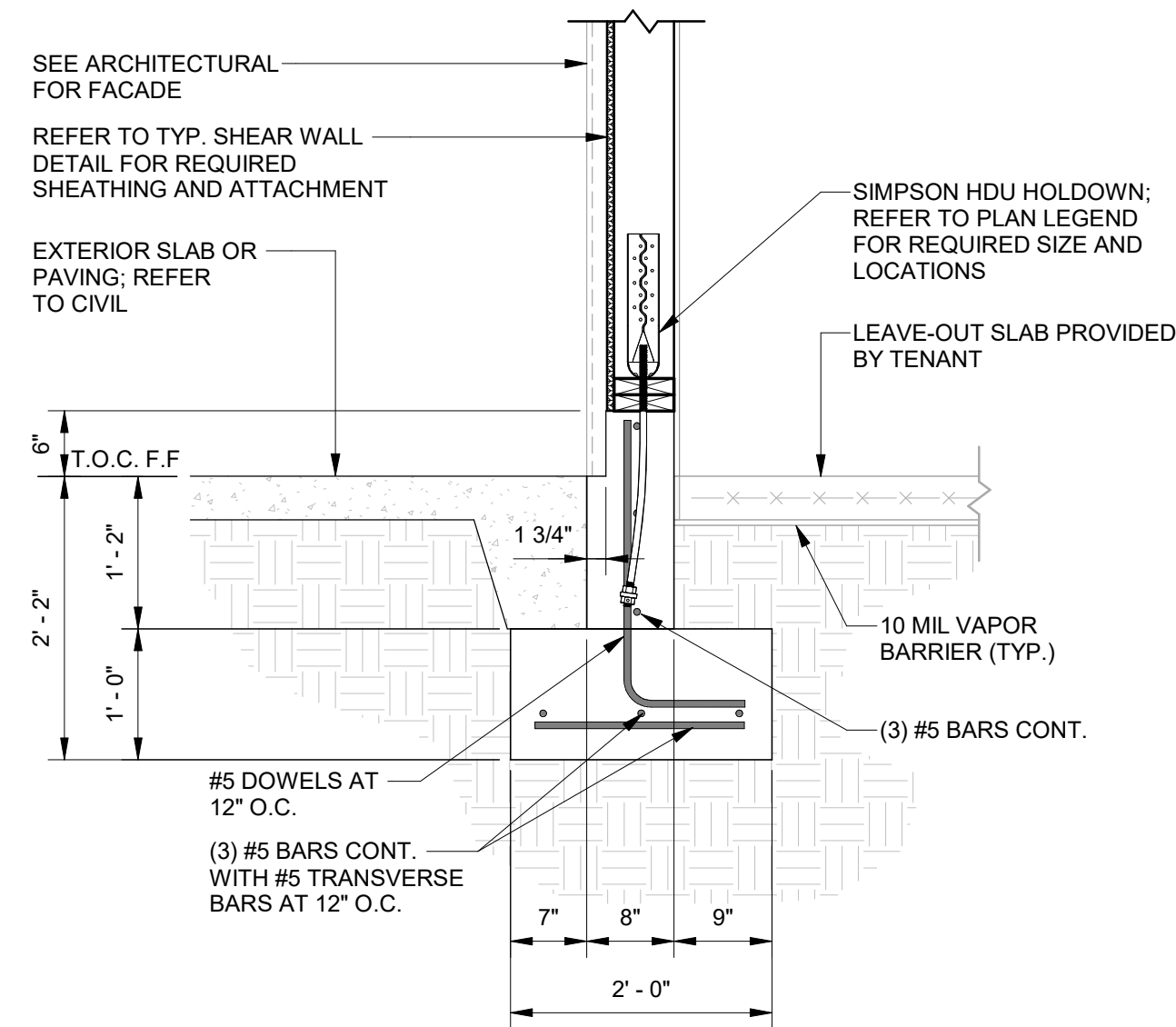
ROOF FRAMING PLAN

S103

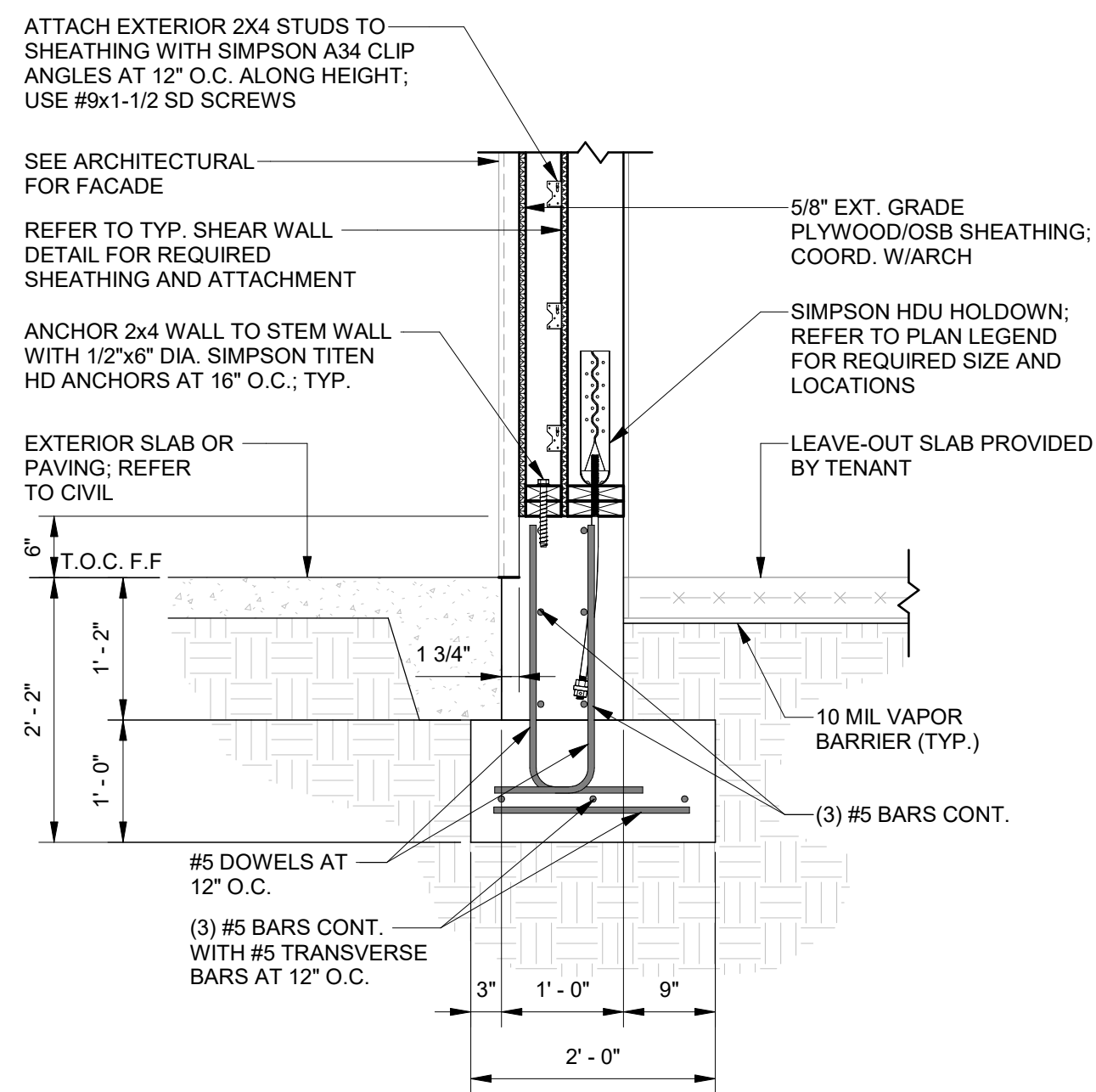


1 ROOF FRAMING PLAN  
Scale: 1/4" = 1'-0"

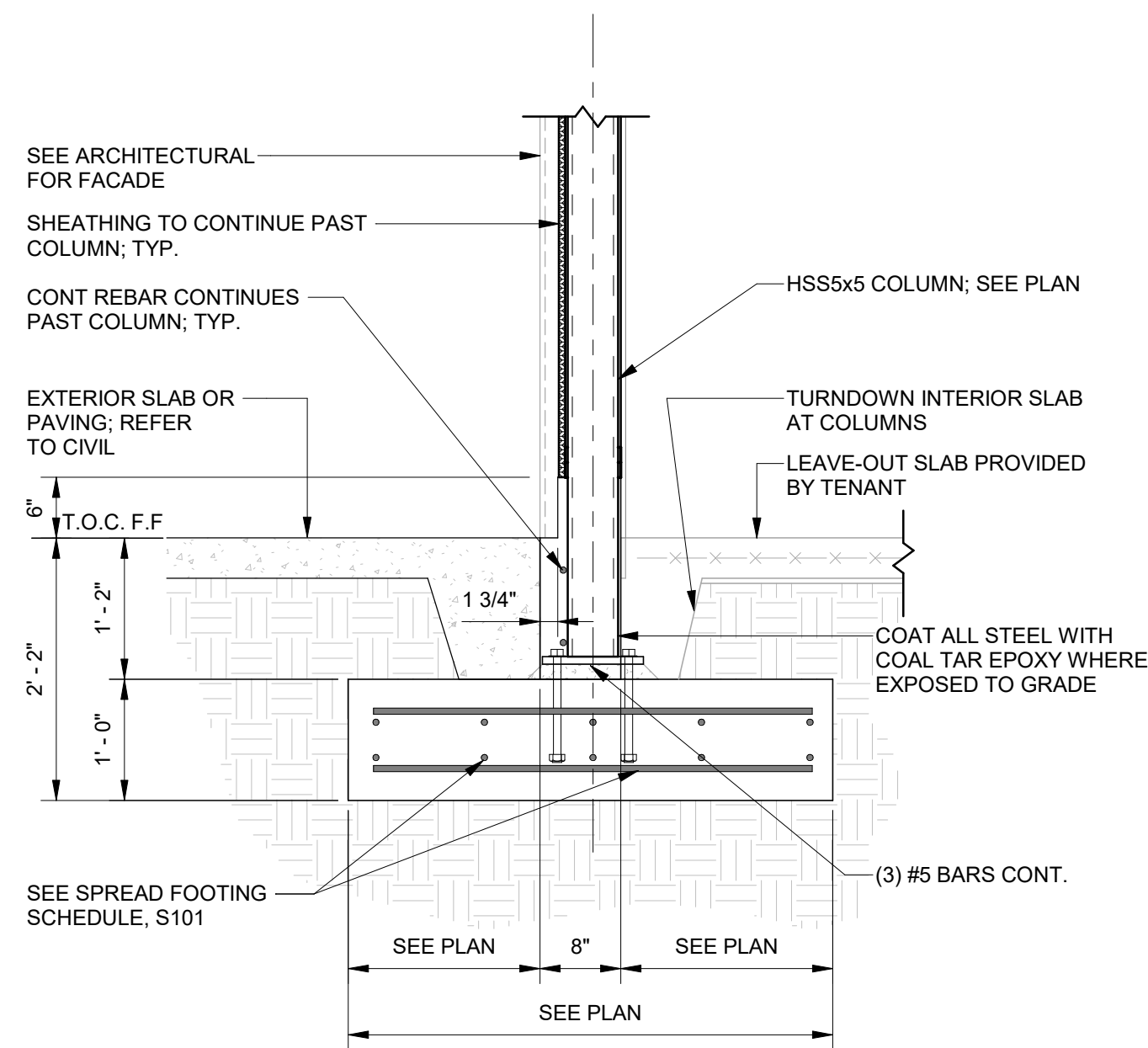
LEGEND	
	PARAPET WALL: 2x6 AT 16" O.C. WITH 2x6 BLOCKING AT 48" O.C. MAX. REFER TO TYPICAL SHEAR WALL DETAIL, S301
	2x4 STUD WALL AT 16" O.C.; TYP. WHERE SHOWN ON ARCH



**1 CONC. STEM WALL W/CURB**  
Scale: 3/4" = 1'-0"



2 CONC. STEM WALL AT DOUBLE WALL  
Scale: 3/4" = 1'-0"



### 3 LOWER FOOTING AT COLUMNS

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[illegible]

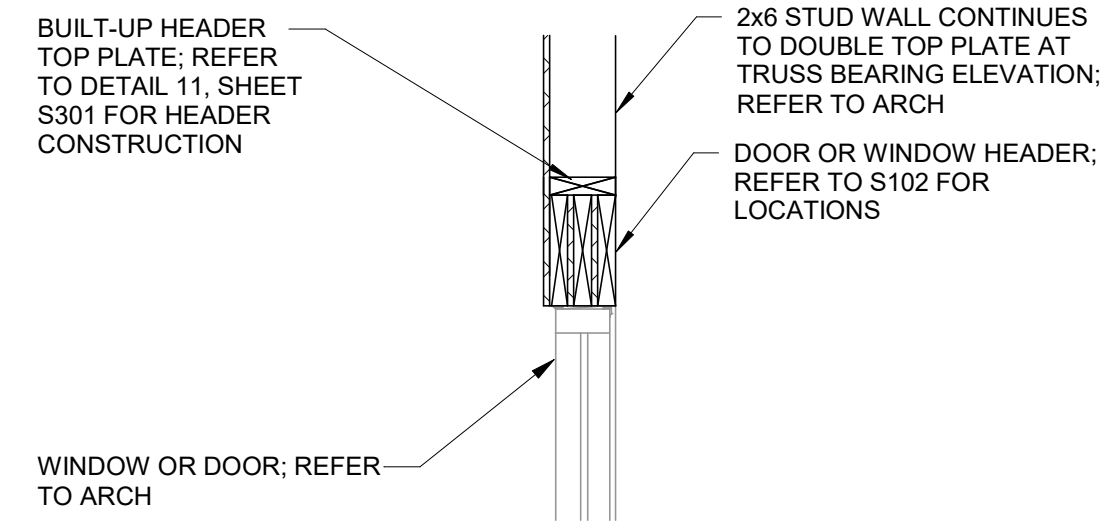
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## FOUNDATION SECTIONS

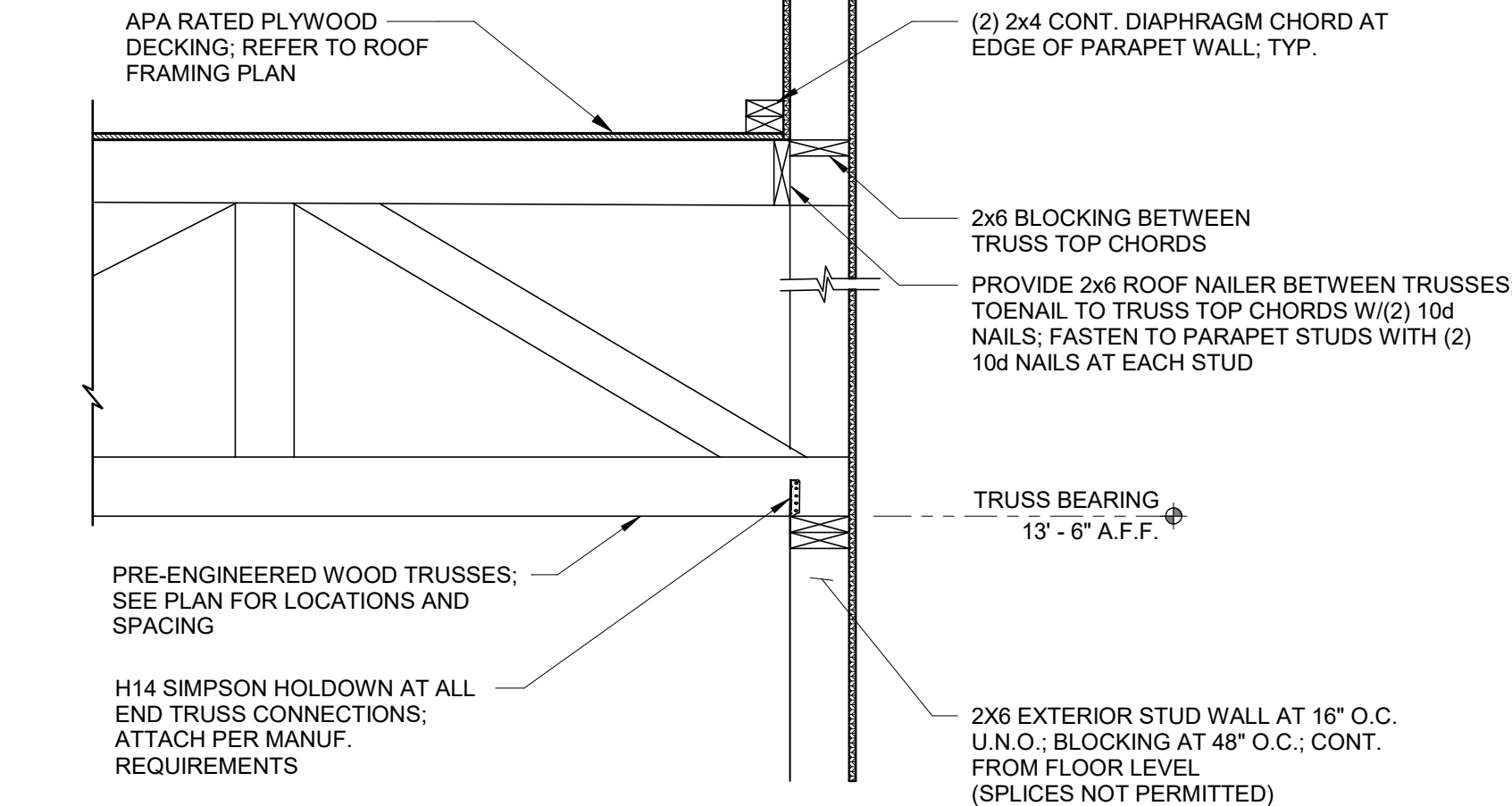
# S201

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**ENGINEERING** (p) 615.244.8170  
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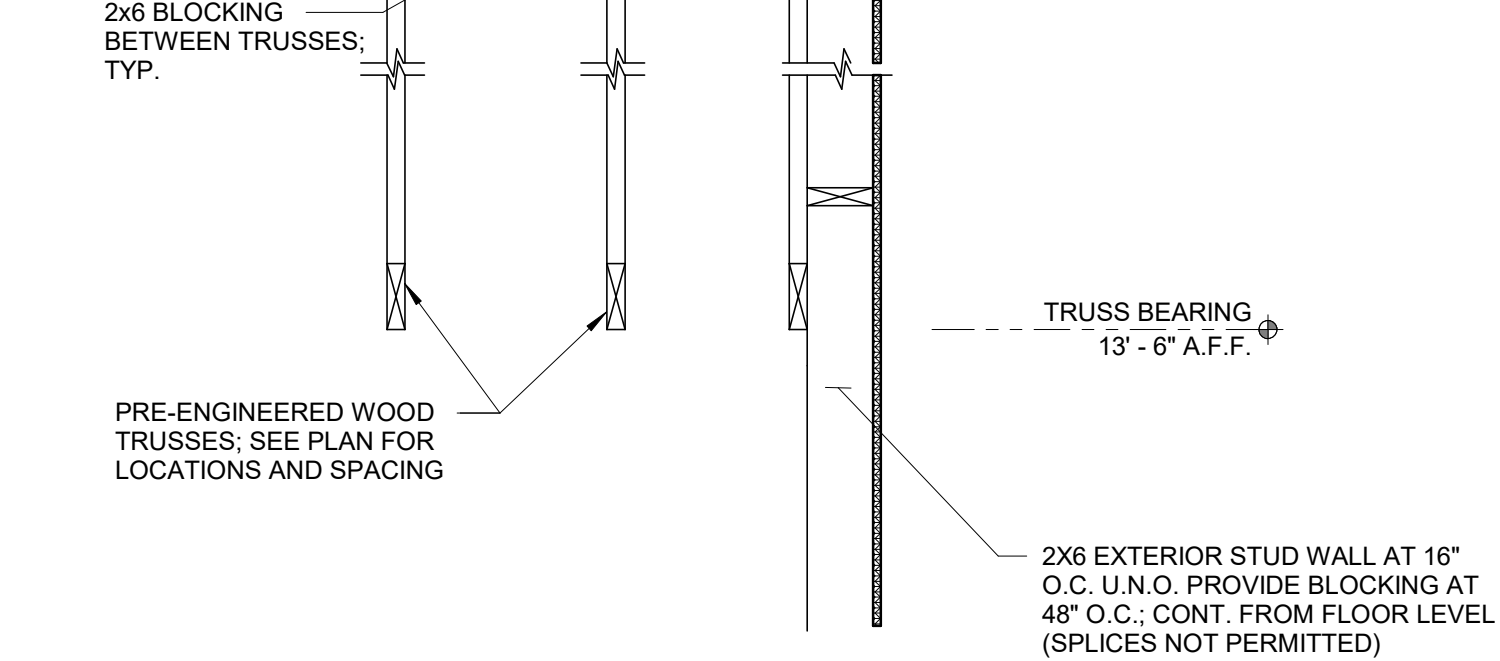




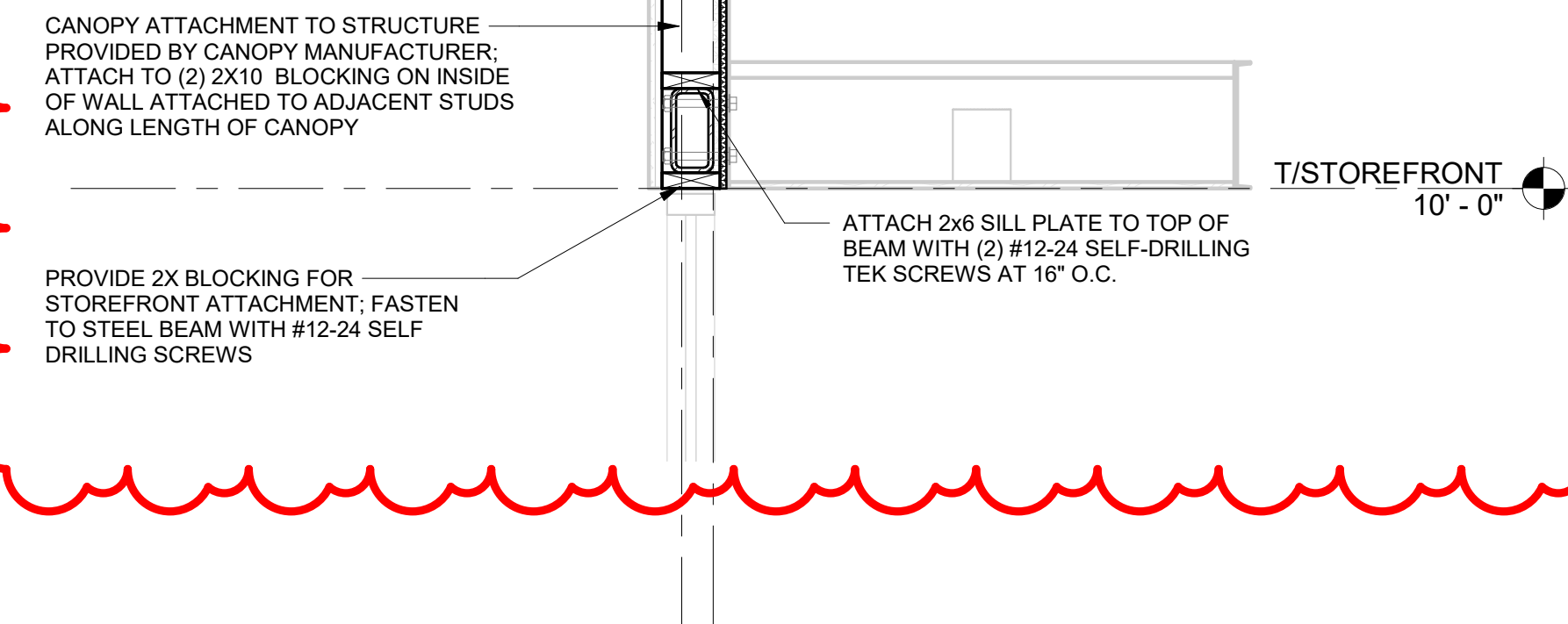
Scale:  $3/4" = 1'-0"$



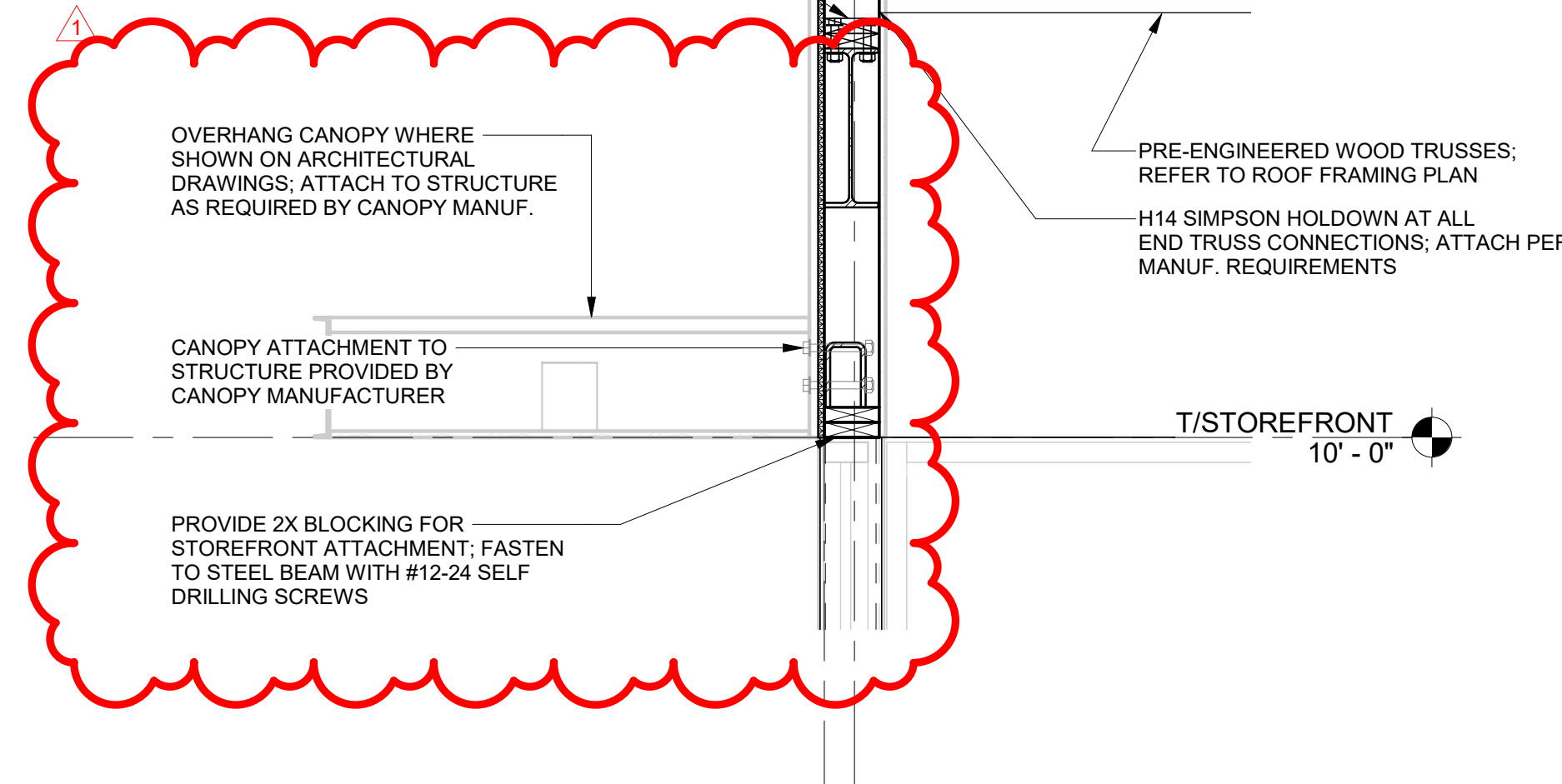
Scale:  $\frac{3}{4}" = 1'-0"$



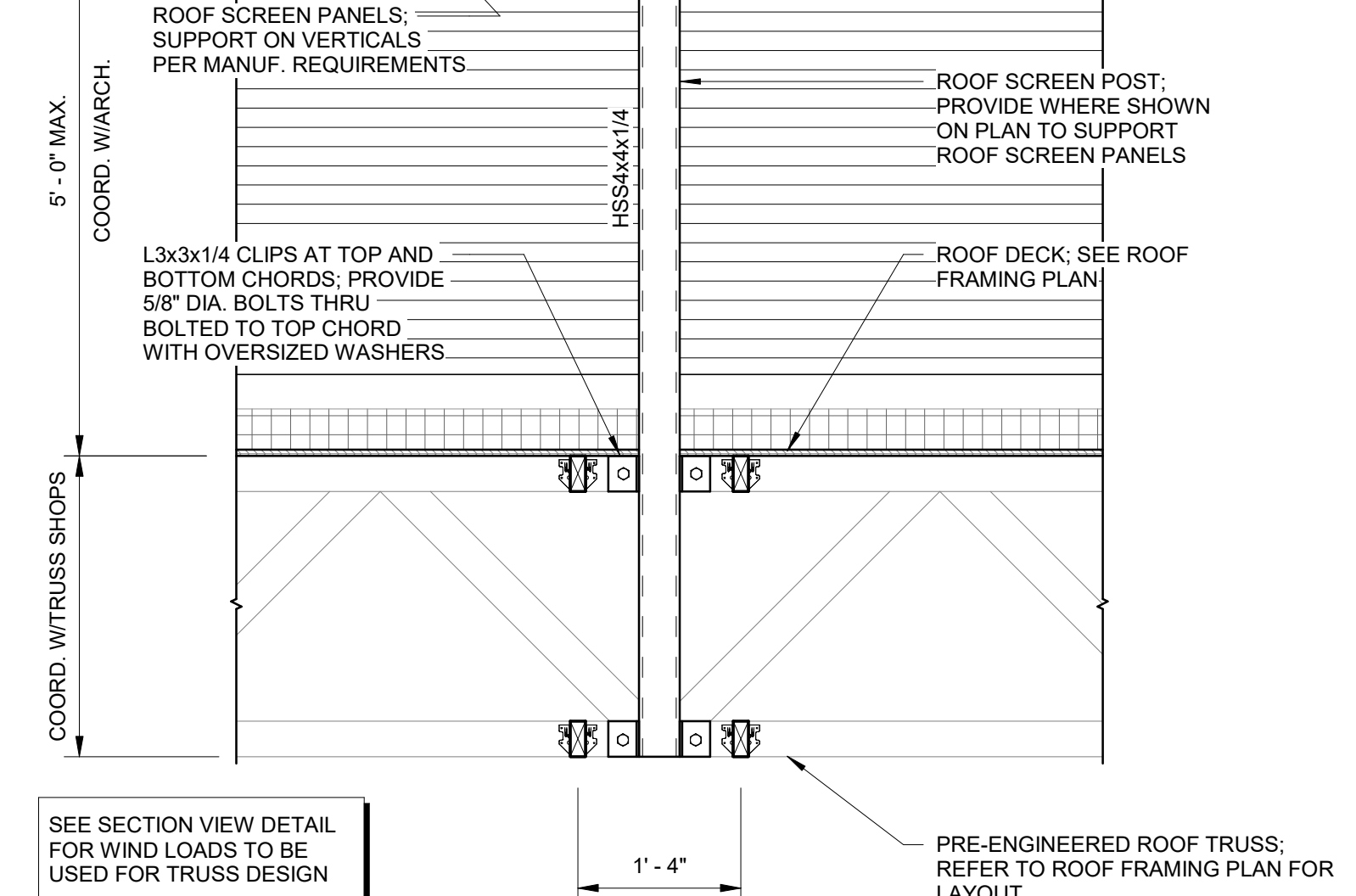
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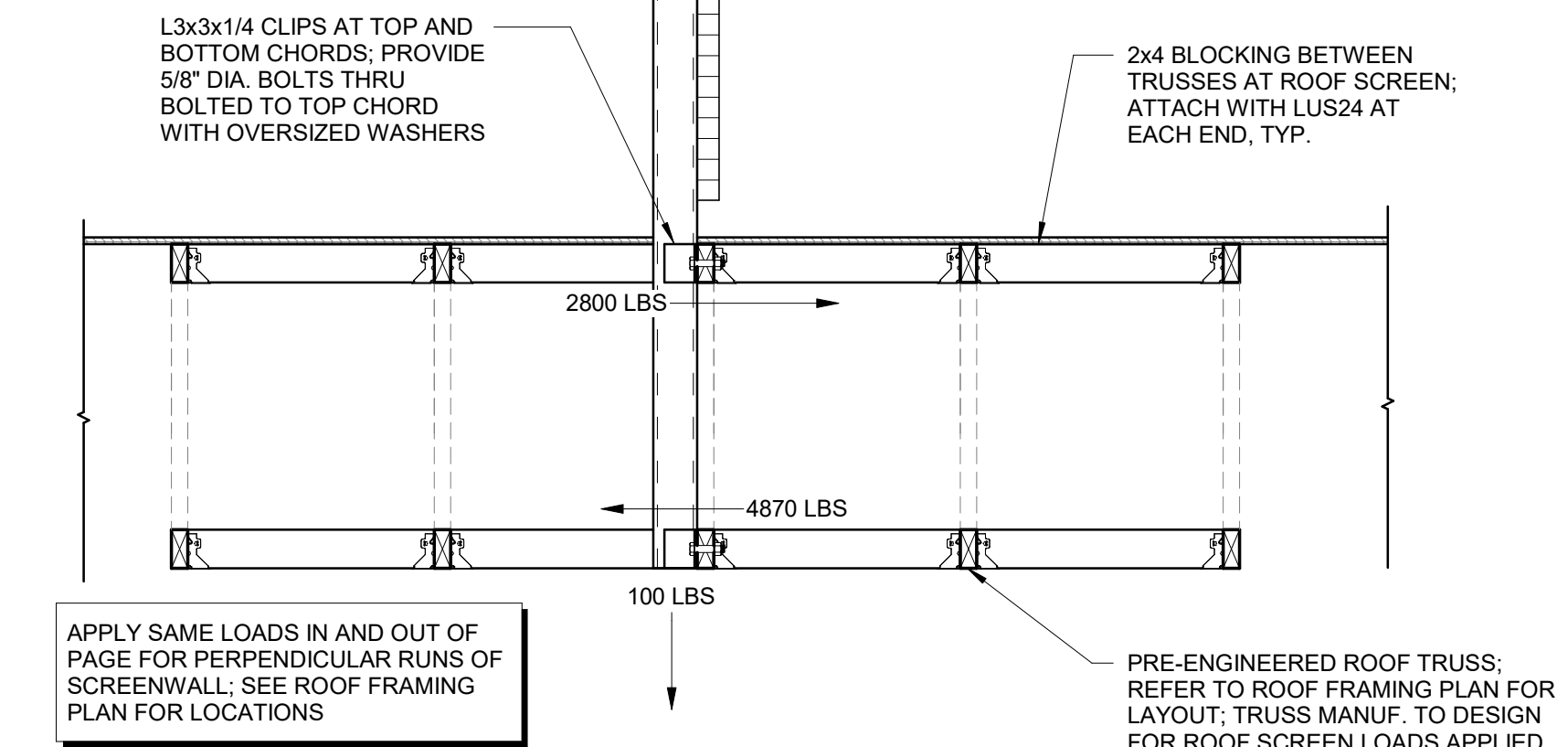
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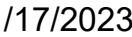
Scale:  $3/4" = 1'-0"$



Scale:  $3/4" = 1'-0$



Scale:  $3/4" = 1'-0$



500 6TH STREET  
SAN ANTONIO, TX 78215

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23331

[illegible]

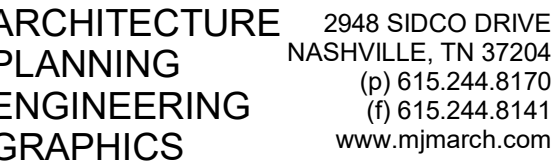
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## FRAMING SECTIONS

# S202







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[illegible]

## SITE DETAILS

# SHEETS



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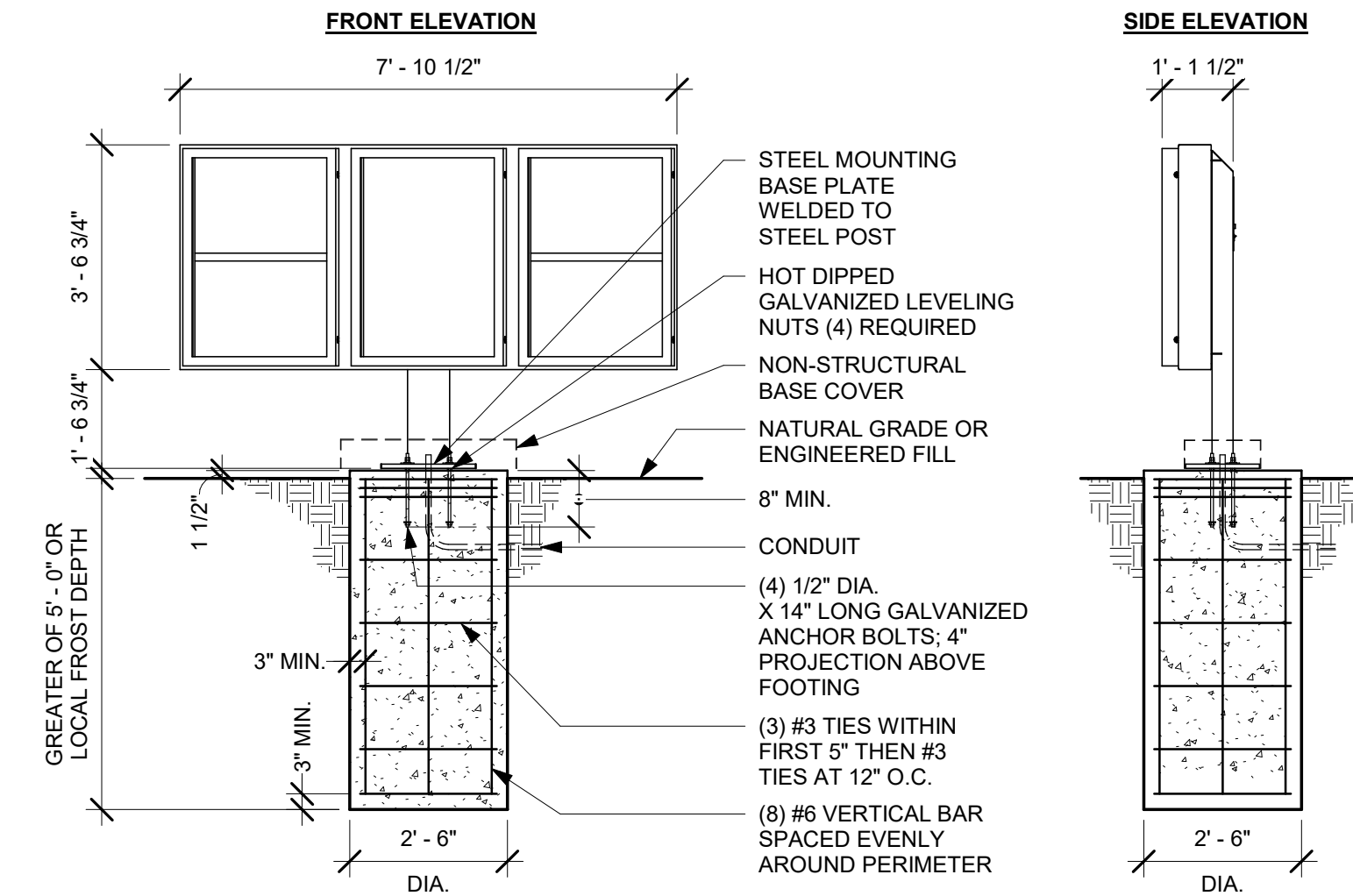
23331

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## SITE DETAILS

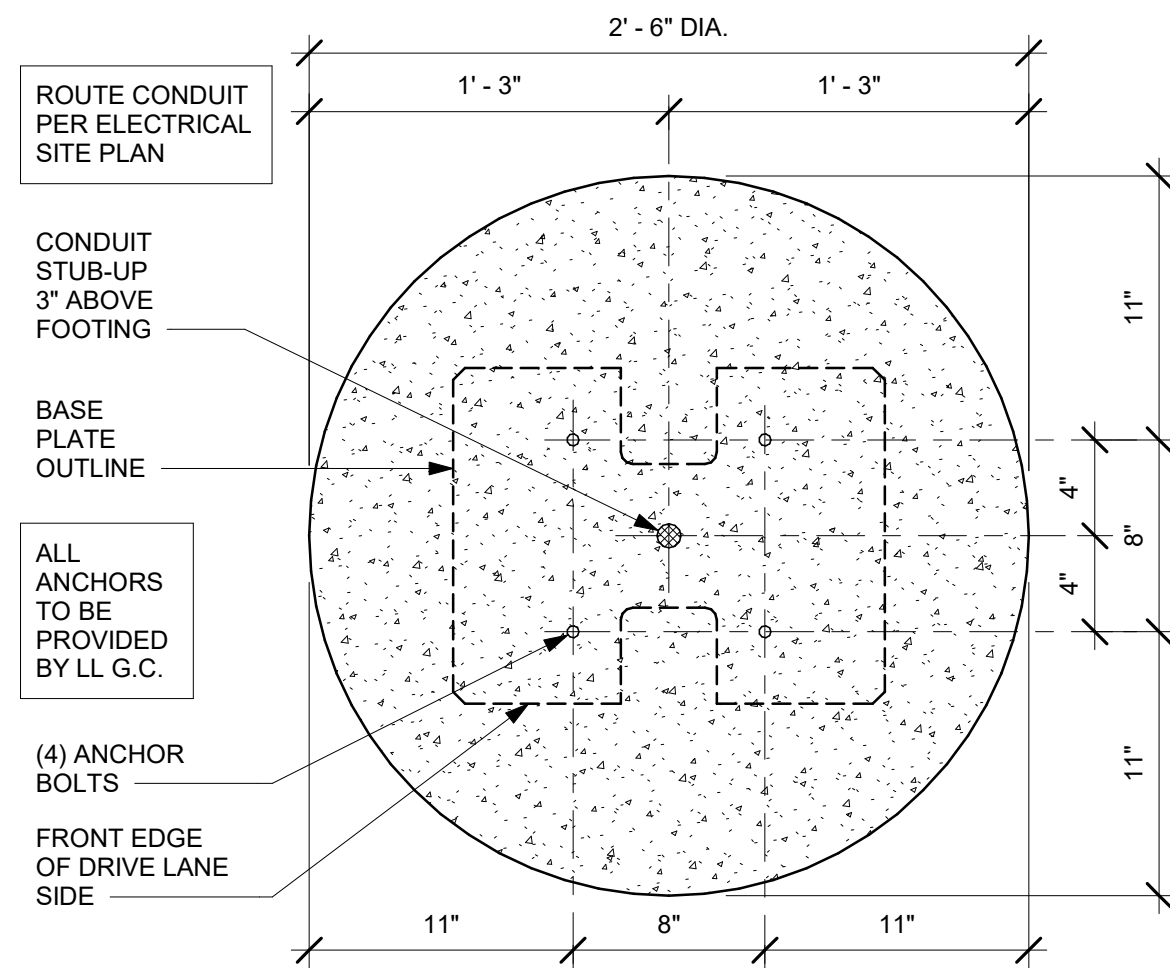
# S303



G.C. RESPONSIBLE FOR FOUNDATIONS AND FOOTINGS ON ALL MENU BOARD AND DRIVE THRU ELEMENTS. SIGNAGE VENDOR TO SUPPLY TEMPLATES AND INSTALL MENU BOARDS ONLY.

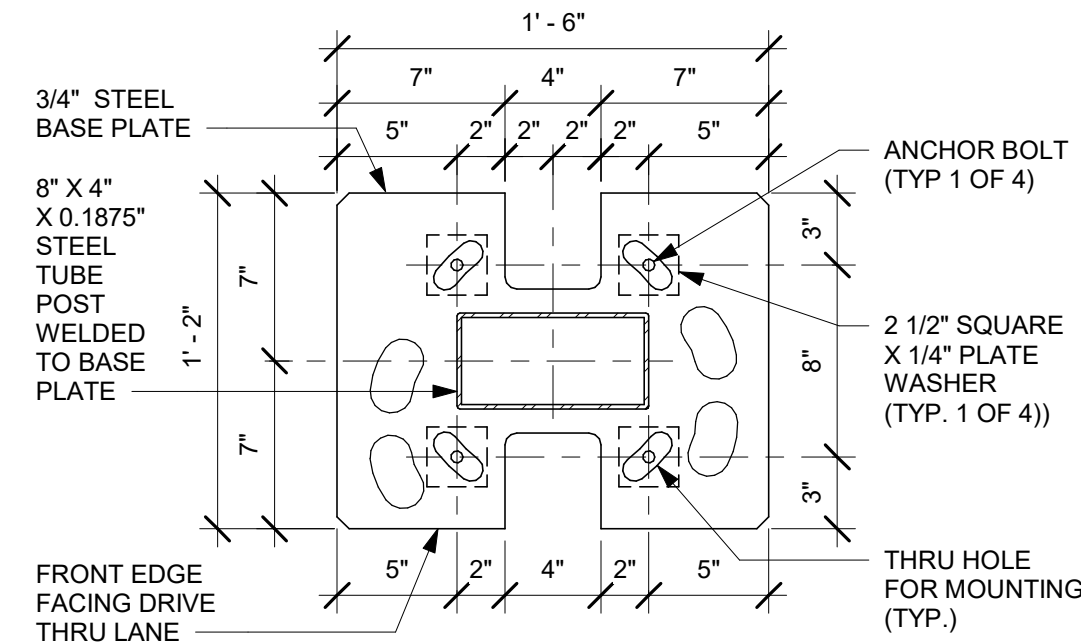
### 1 DT 5-PANEL MENU BOARD GROUND FOOTING

Scale: 3/8" = 1'-0"



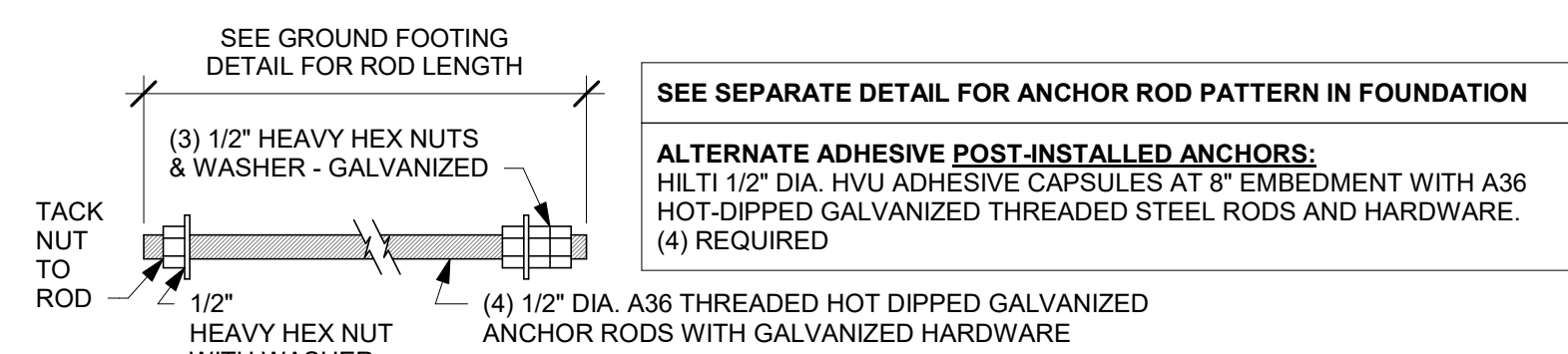
### 2 DTE 5-PANEL MENU BOARD BOLT PATTERN (TOP VIEW)

Scale: 1 1/2" = 1'-0"



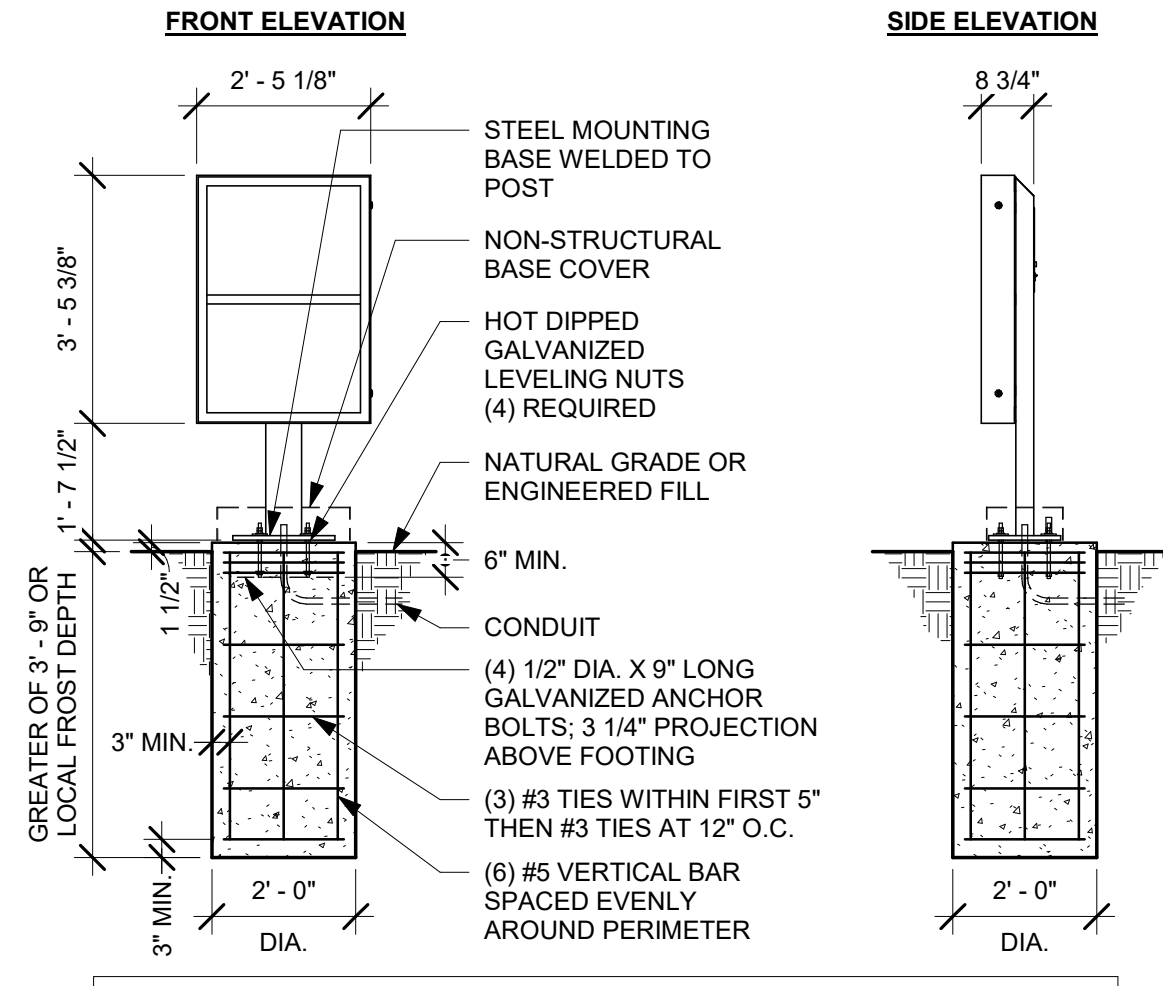
### 3 DTE 5-PANEL MENU BOARD BASE PLATE

Scale: 1 1/2" = 1'-0"



### 4 DTE 5-PANEL MENU BOARD ANCHOR ROD

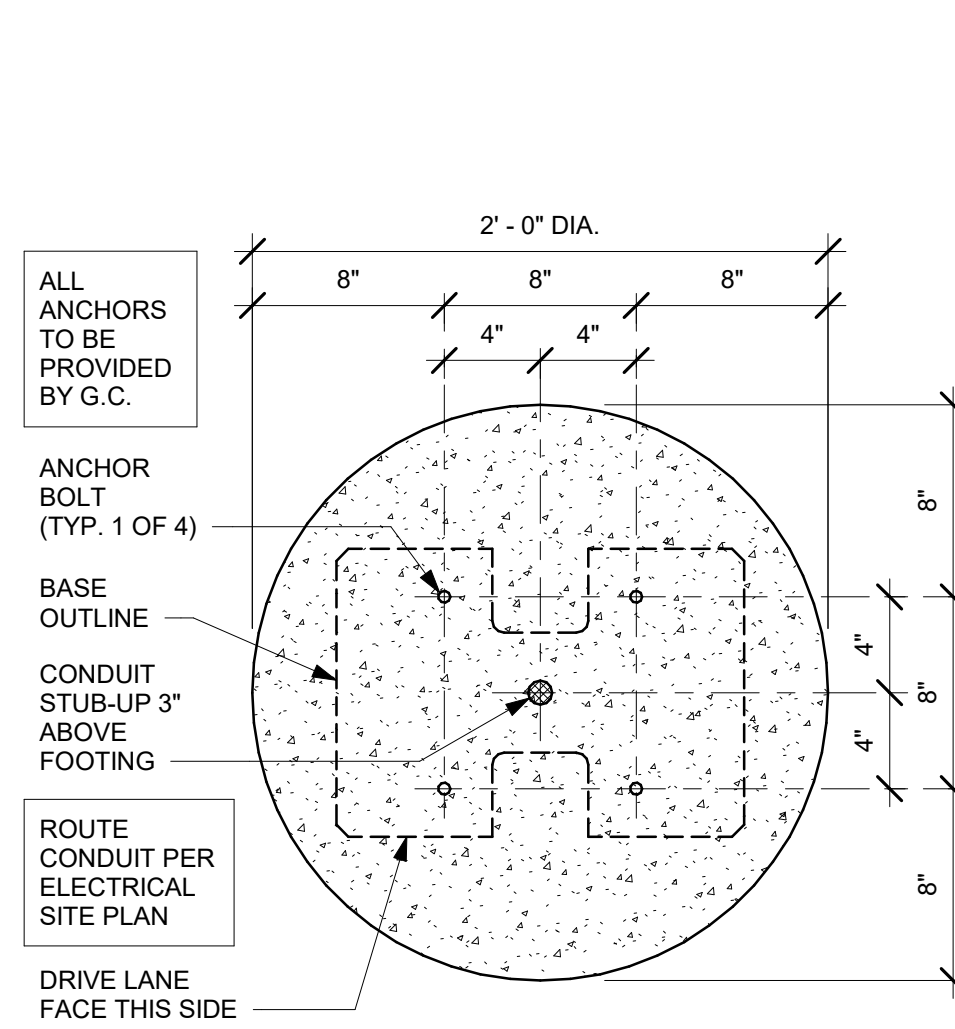
Scale: 3" = 1'-0"



G.C. RESPONSIBLE FOR FOUNDATIONS AND FOOTINGS ON ALL MENU BOARD AND DRIVE THRU ELEMENTS. SIGNAGE VENDOR TO SUPPLY TEMPLATES AND INSTALL MENU BOARDS ONLY.

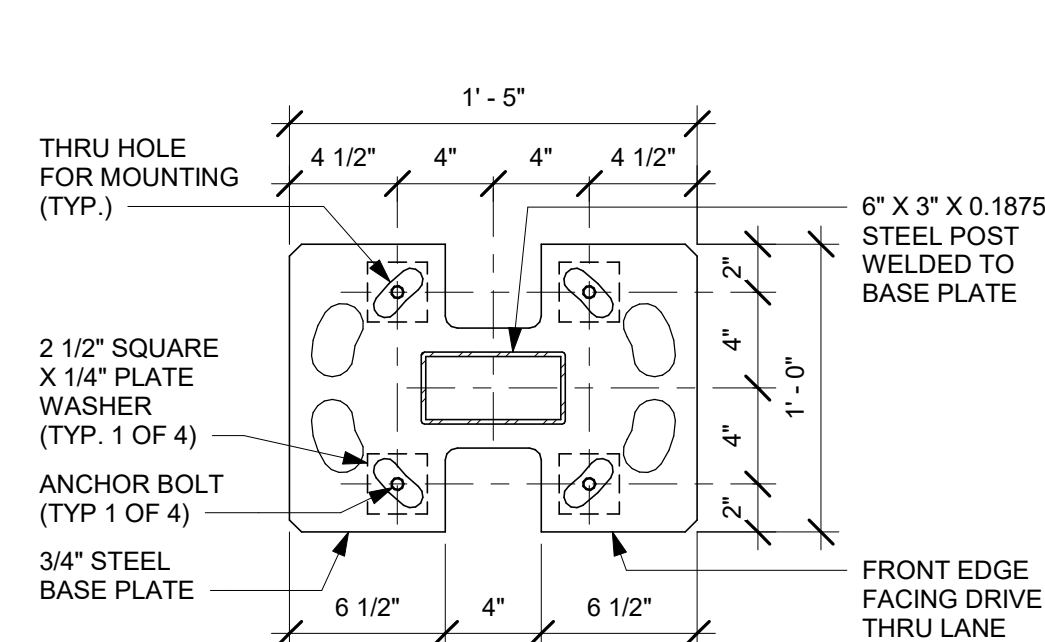
### 5 DTE PRE-MENU GROUND FOOTING

Scale: 3/8" = 1'-0"



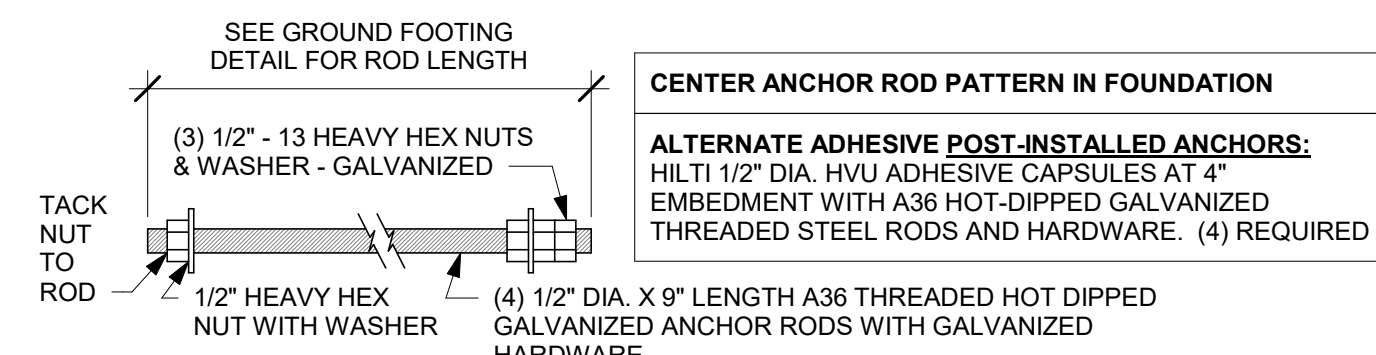
### 6 DTE PRE-MENU BOLT PATTERN (TOP VIEW)

Scale: 1 1/2" = 1'-0"



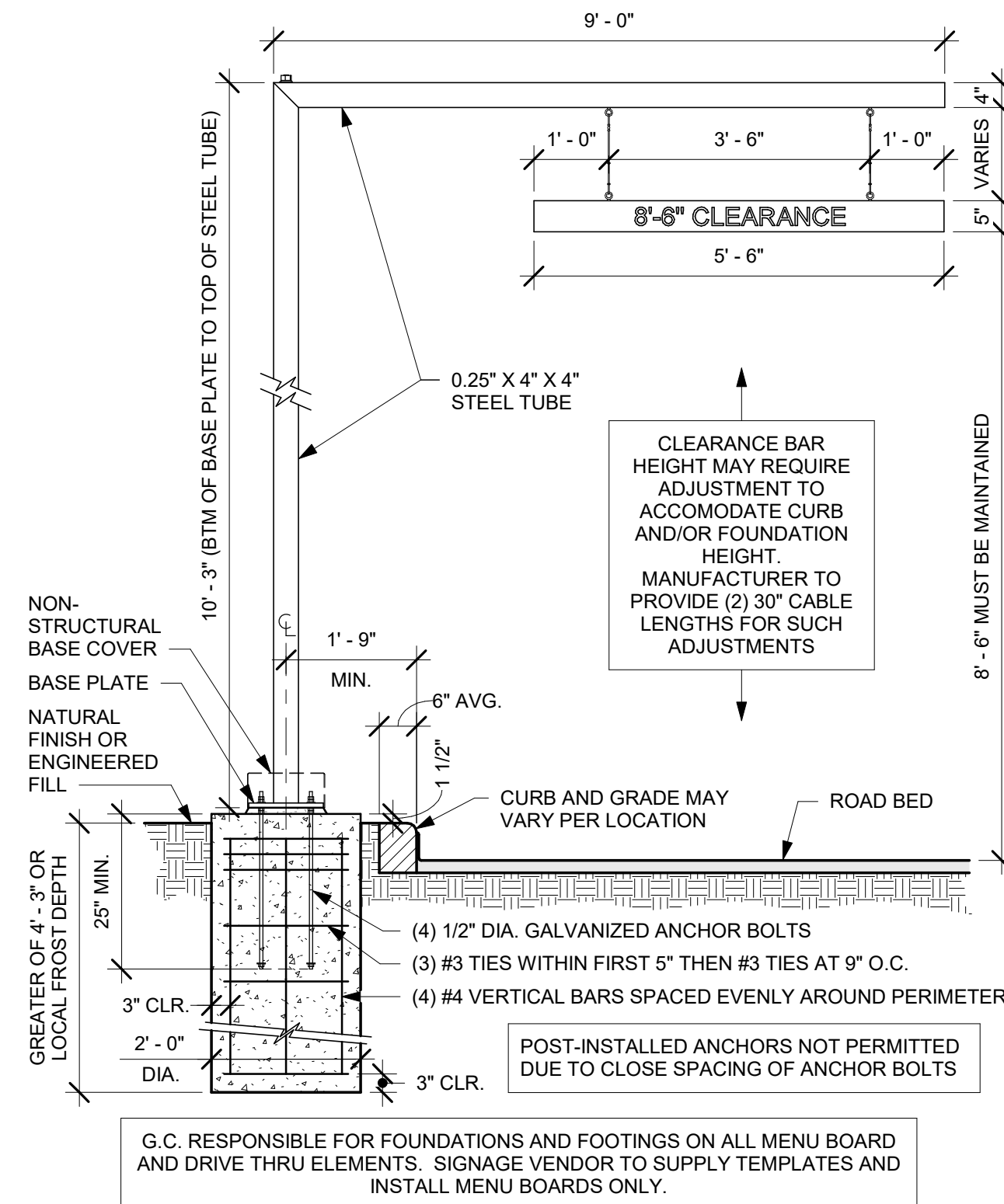
### 7 DTE PRE-MENU BASE PLATE

Scale: 1 1/2" = 1'-0"



### 8 DTE PRE-MENU ANCHOR ROD

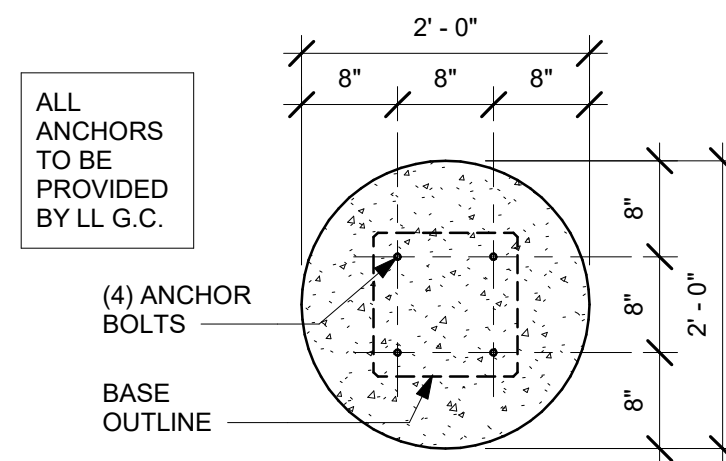
Scale: 3" = 1'-0"



G.C. RESPONSIBLE FOR FOUNDATIONS AND FOOTINGS ON ALL MENU BOARD AND DRIVE THRU ELEMENTS. SIGNAGE VENDOR TO SUPPLY TEMPLATES AND INSTALL MENU BOARDS ONLY.

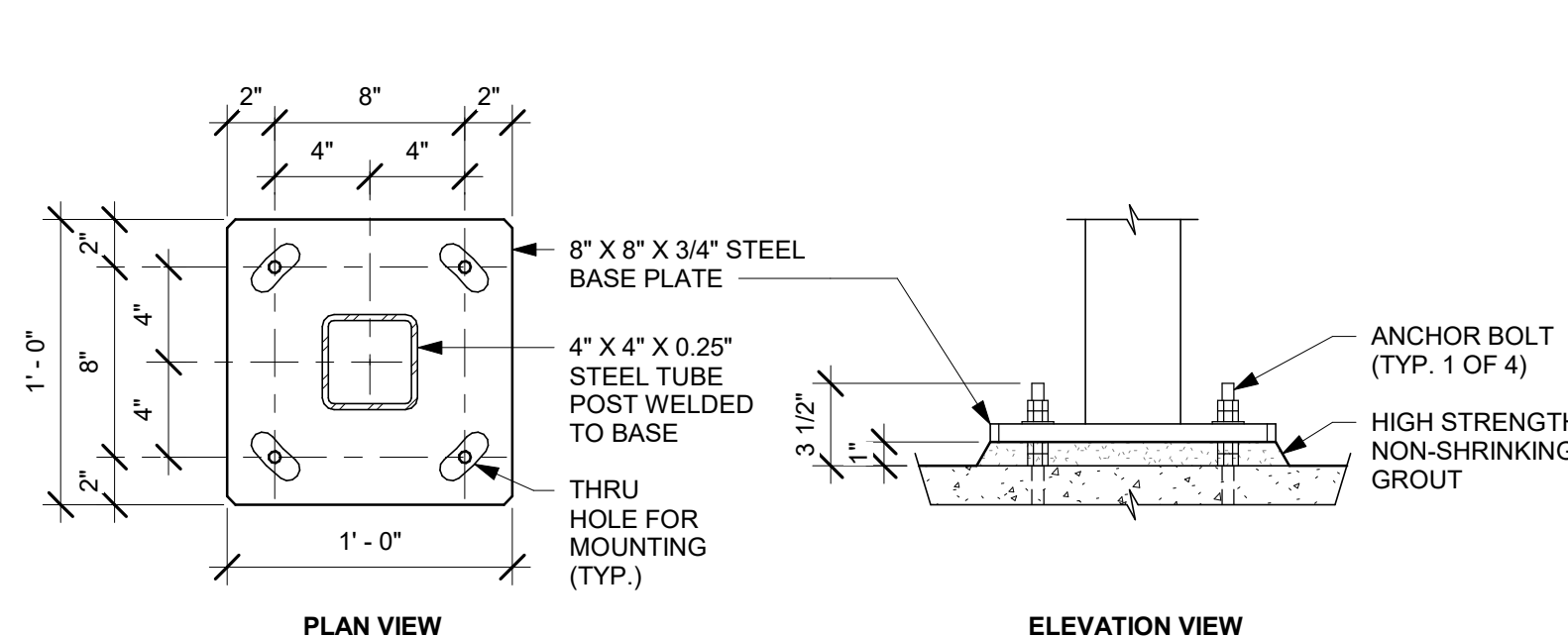
### 9 DTE SIGN - CLEARANCE BAR - GROUND FOOTING

Scale: 1/2" = 1'-0"



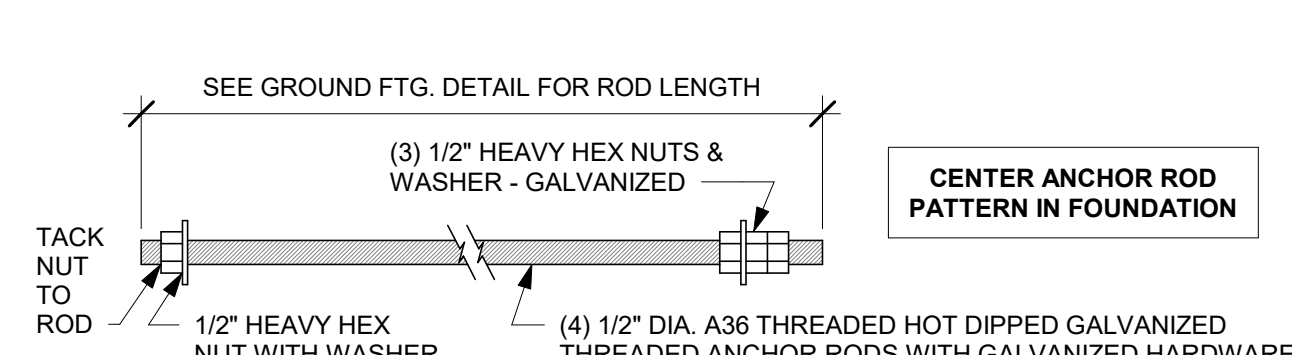
### 10 DTE CLEARANCE BAR BOLT PATTERN (TOP VIEW)

Scale: 3/4" = 1'-0"



### 11 DTE CLEARANCE BAR BASE PLATE

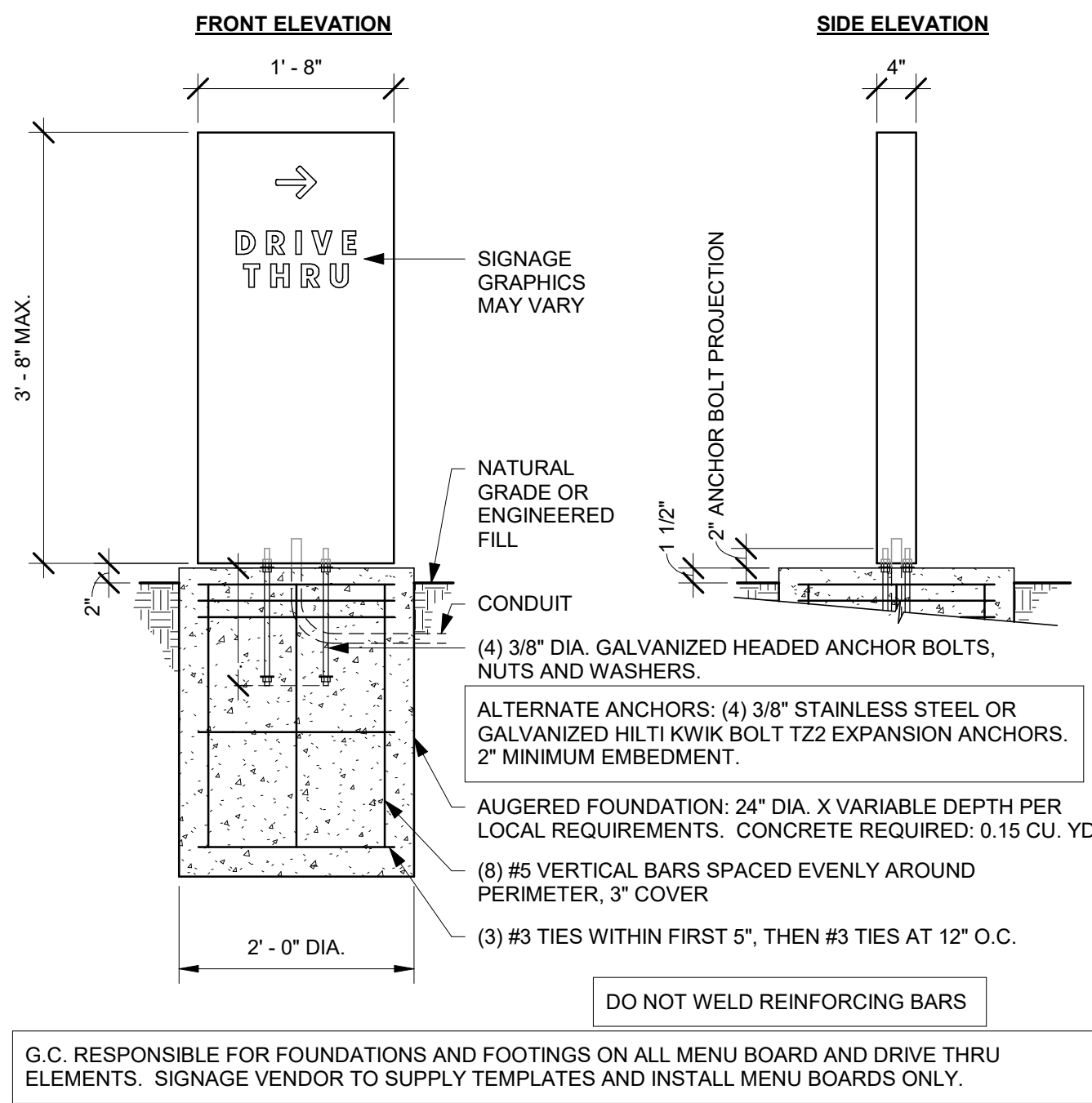
Scale: 1 1/2" = 1'-0"



### 12 DTE CLEARANCE BAR ANCHOR ROD

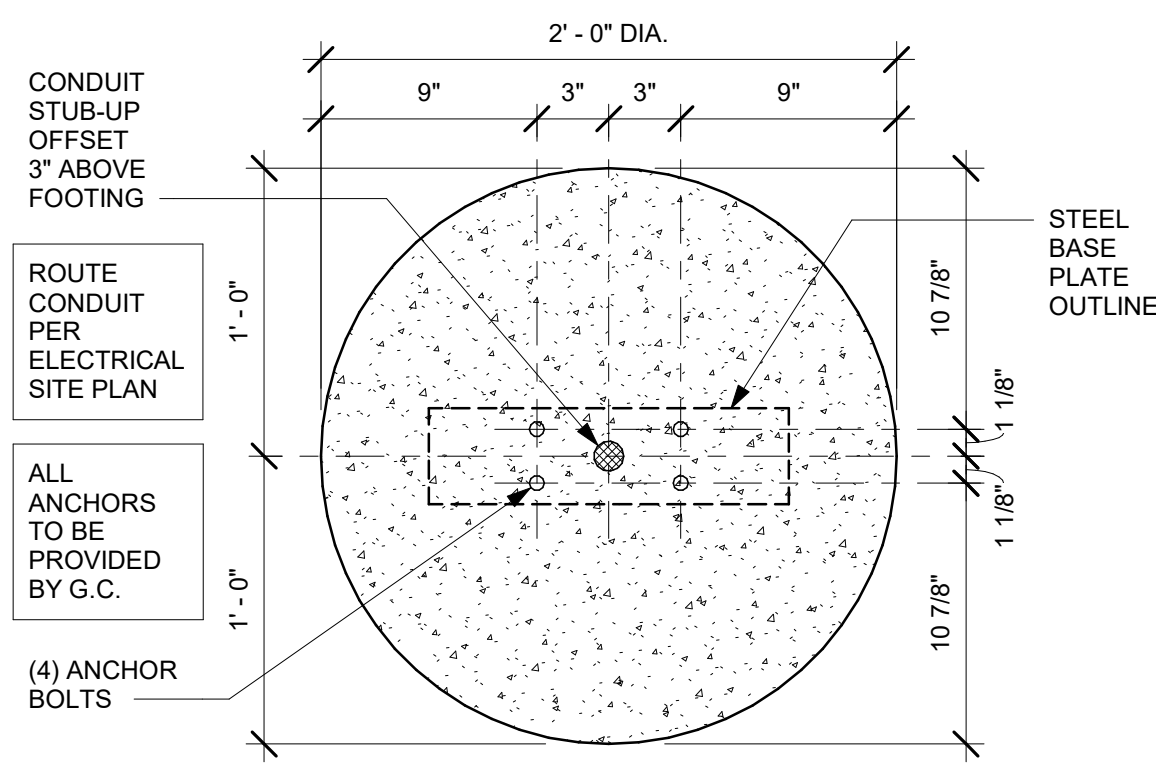
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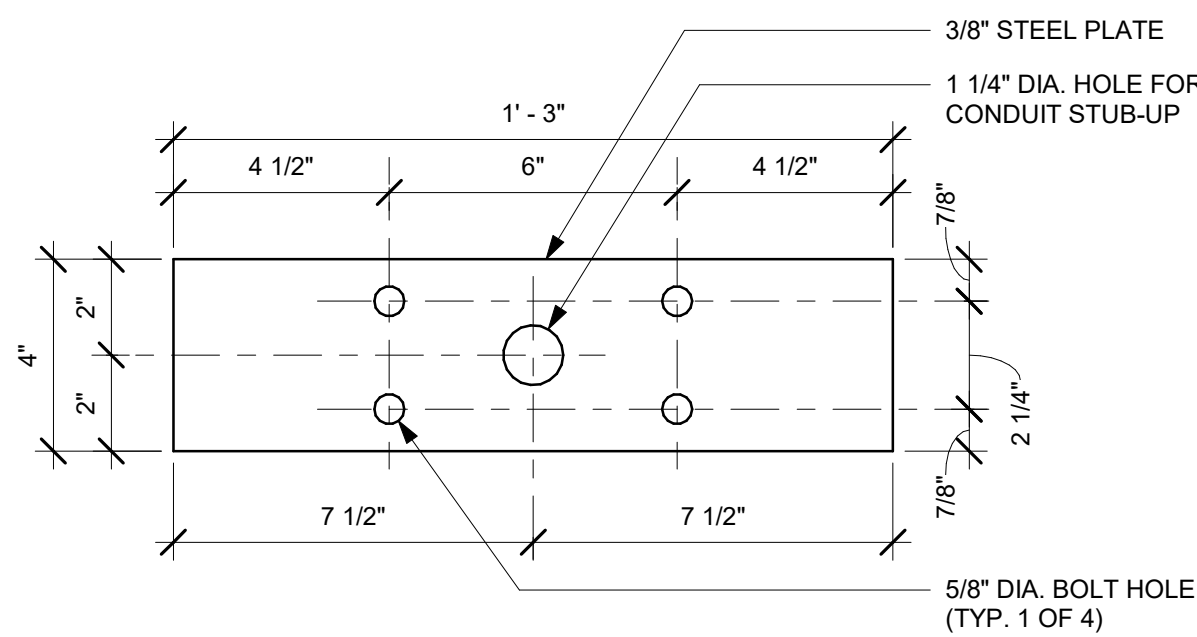
1 DTE DIRECTIONAL SIGNAGE GROUND FOOTING

Scale: 3/4" = 1'-0"



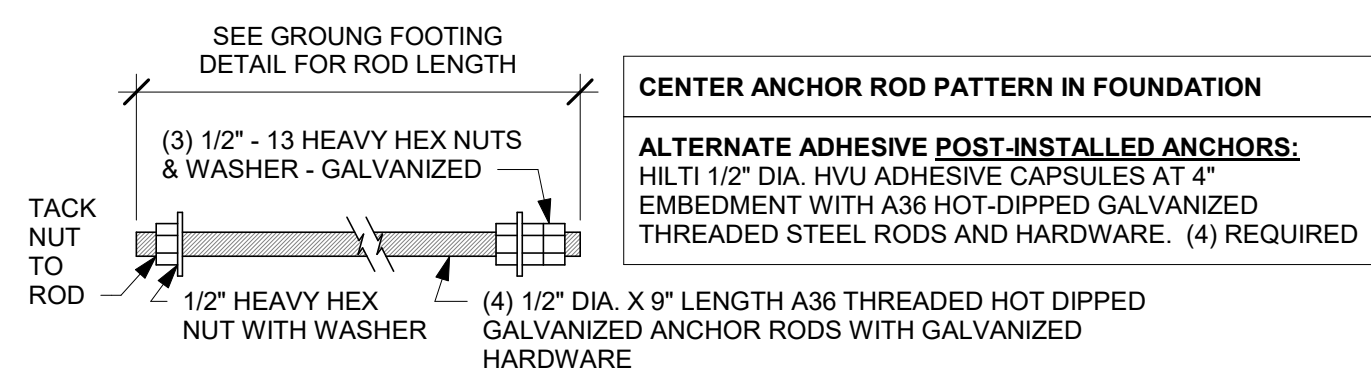
2 DTE DIRECTIONAL SIGNAGE BOLT PATTERN (TOP VIEW)

Scale: 1 1/2" = 1'-0"



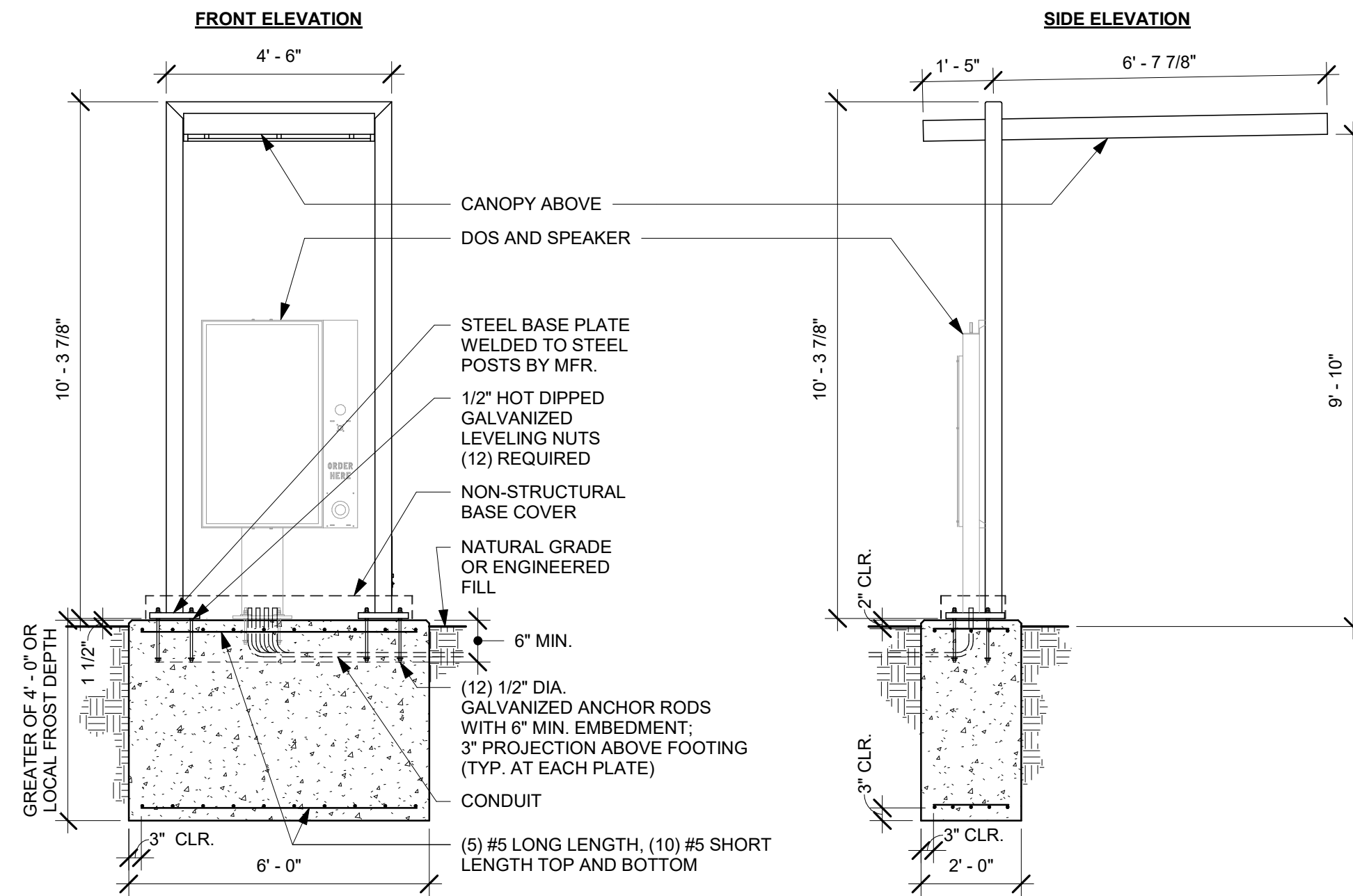
3 DTE DIRECTIONAL SIGNAGE BASE PLATE

Scale: 3" = 1'-0"



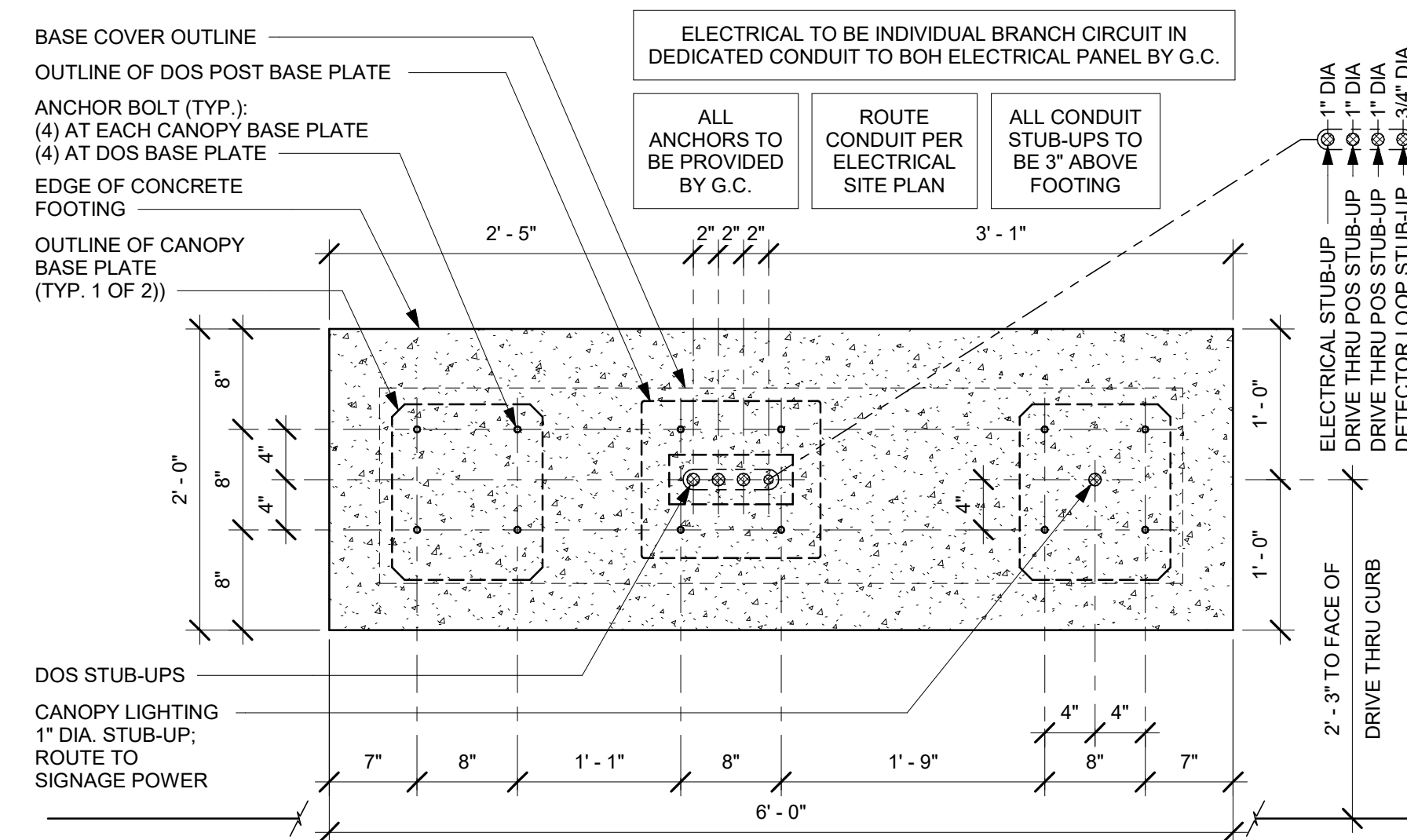
4 DTE DOS POST ANCHOR ROD

Scale: 3" = 1'-0"



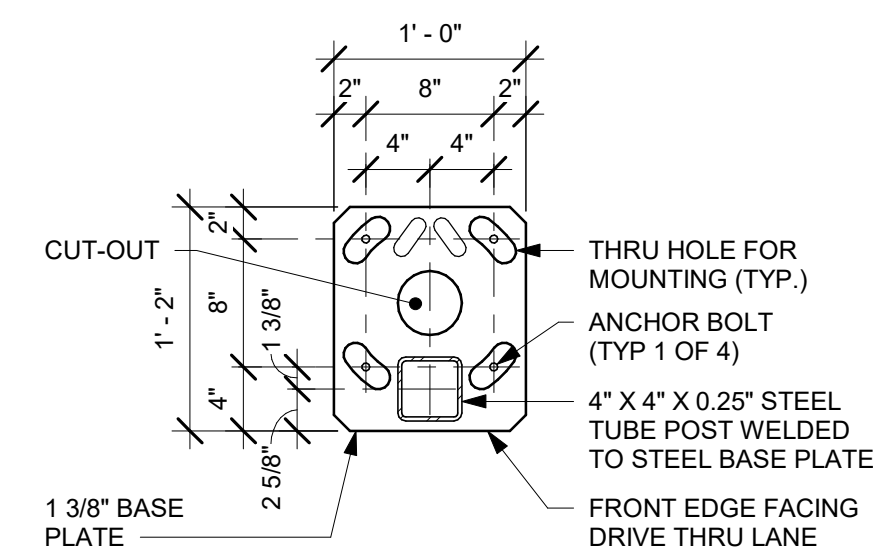
5 DTE DOS CANOPY GROUND FOOTING

Scale: 3/8" = 1'-0"



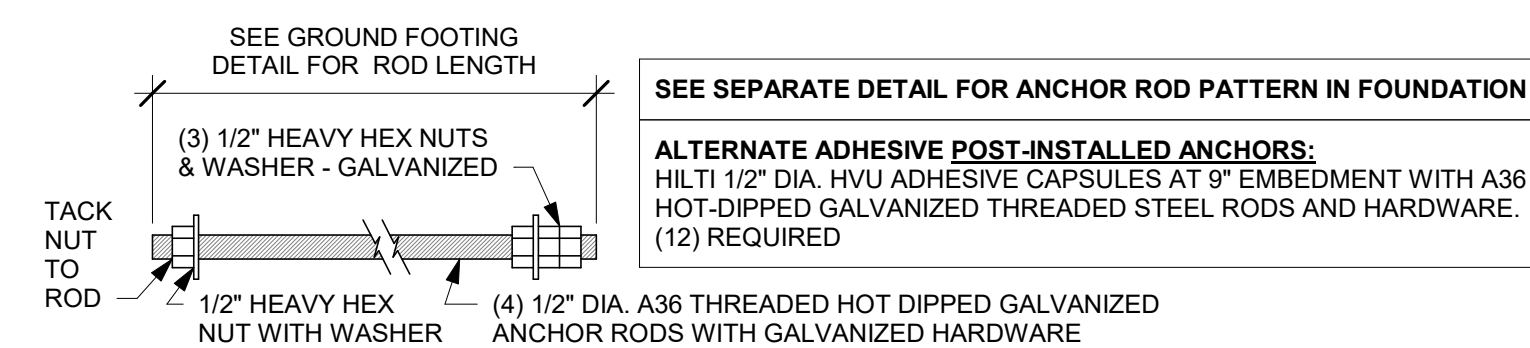
6 DTE DOS CANOPY BOLT PATTERN (TOP VIEW)

Scale: 1" = 1'-0"



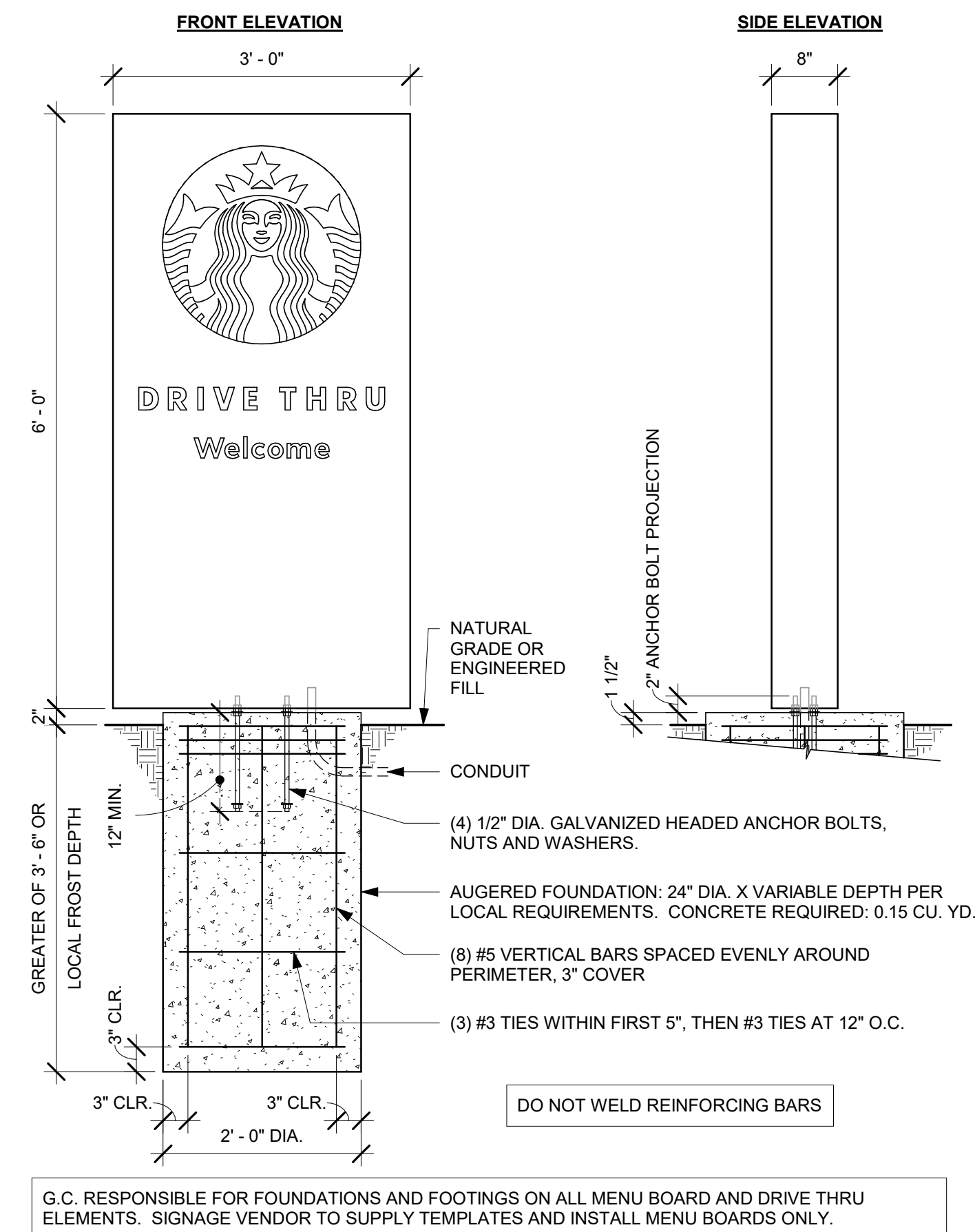
7 DTE DOS CANOPY BASE PLATE

Scale: 1" = 1'-0"



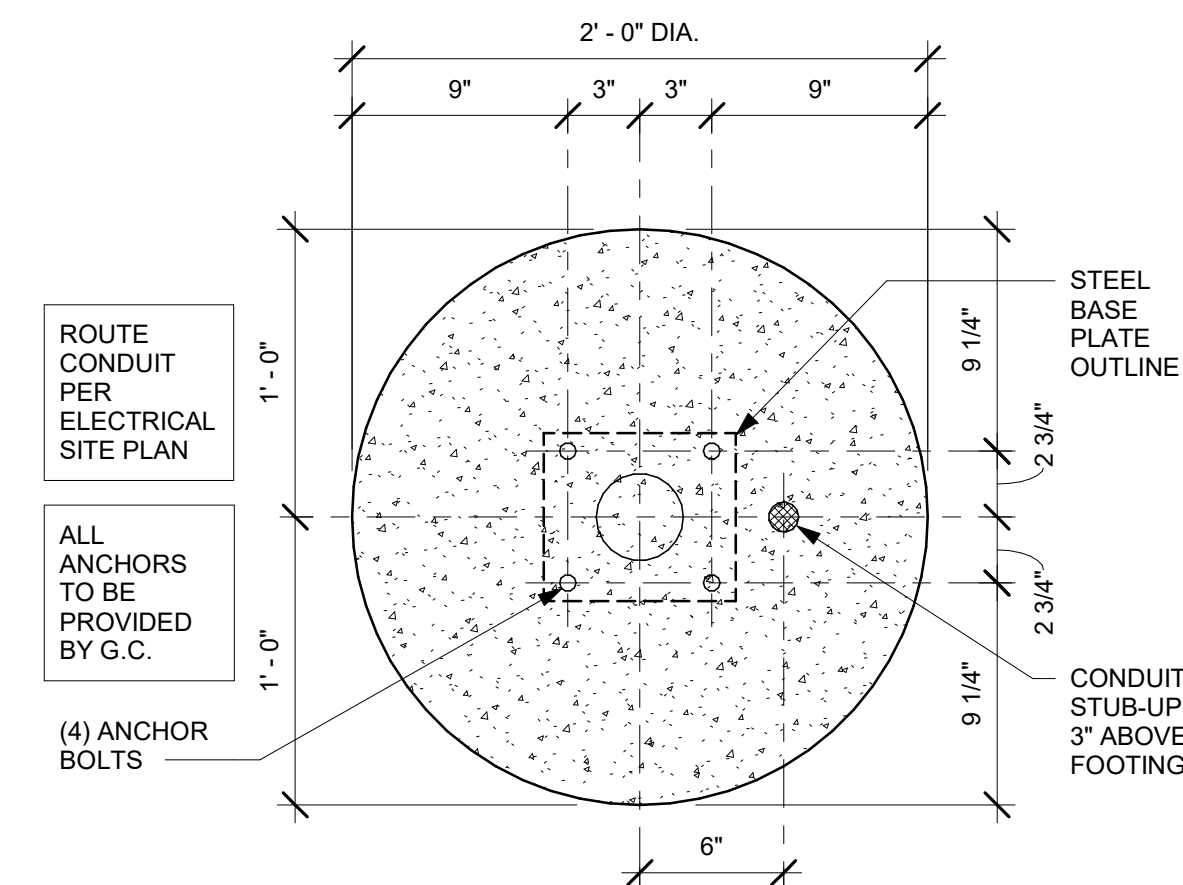
8 REAR UTILITY SCREEN FOUNDATION

Scale: 3" = 1'-0"



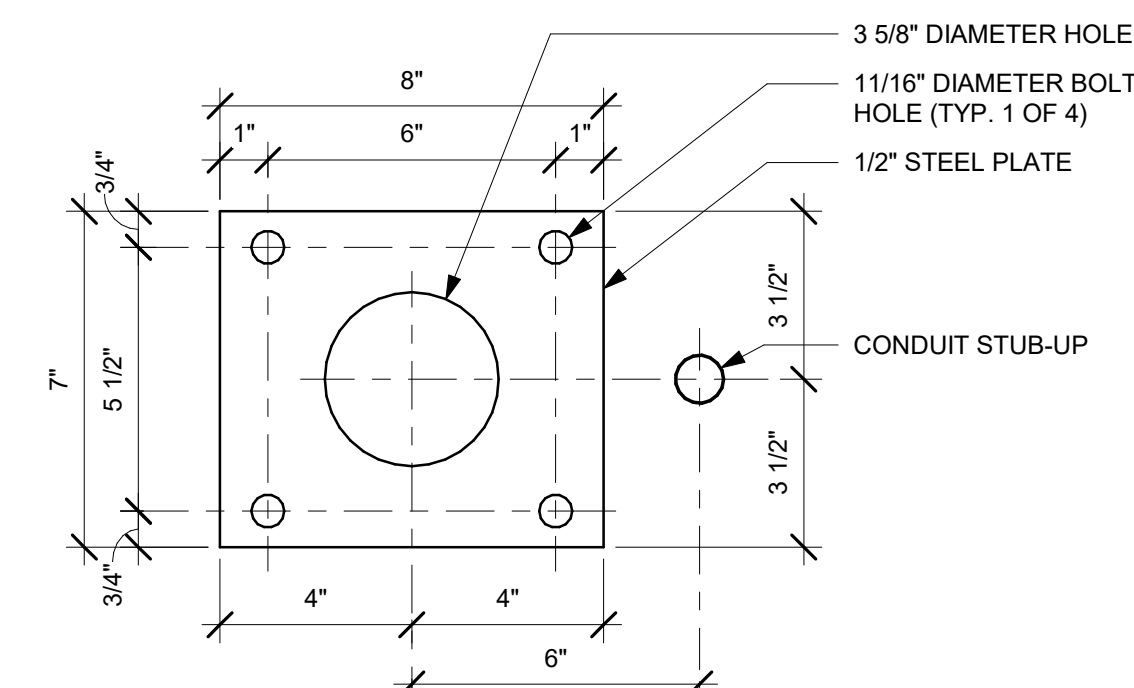
9 DTE MONUMENT SIGNAGE GROUND FOOTING

Scale: 3/4" = 1'-0"



10 DTE MONUMENT SIGNAGE BOLT PATTERN (TOP VIEW)

Scale: 1 1/2" = 1'-0"



11 DTE DOS CANOPY ANCHOR ROD

Scale: 3" = 1'-0"

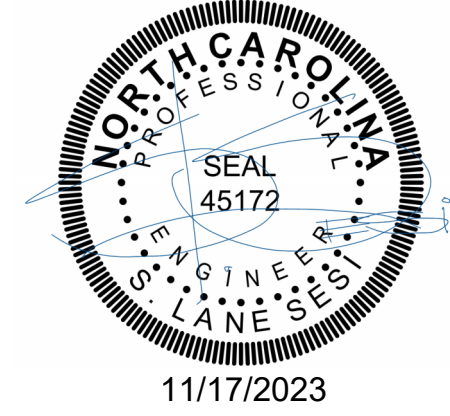
12 DTE MONUMENT BASE PLATE

Scale: 1/2" = 1'-0"



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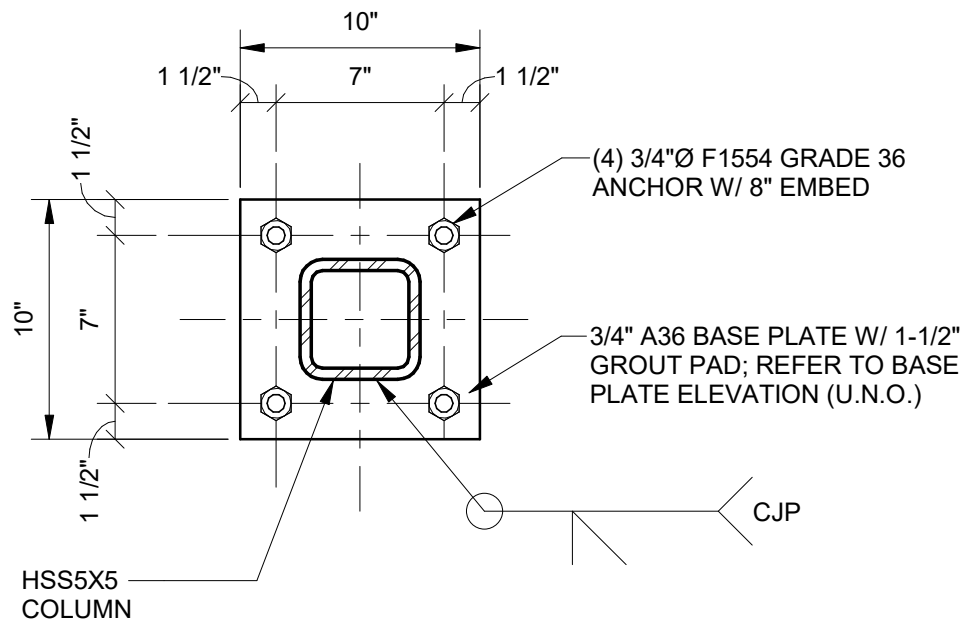
NO.	DESCRIPTION	DATE

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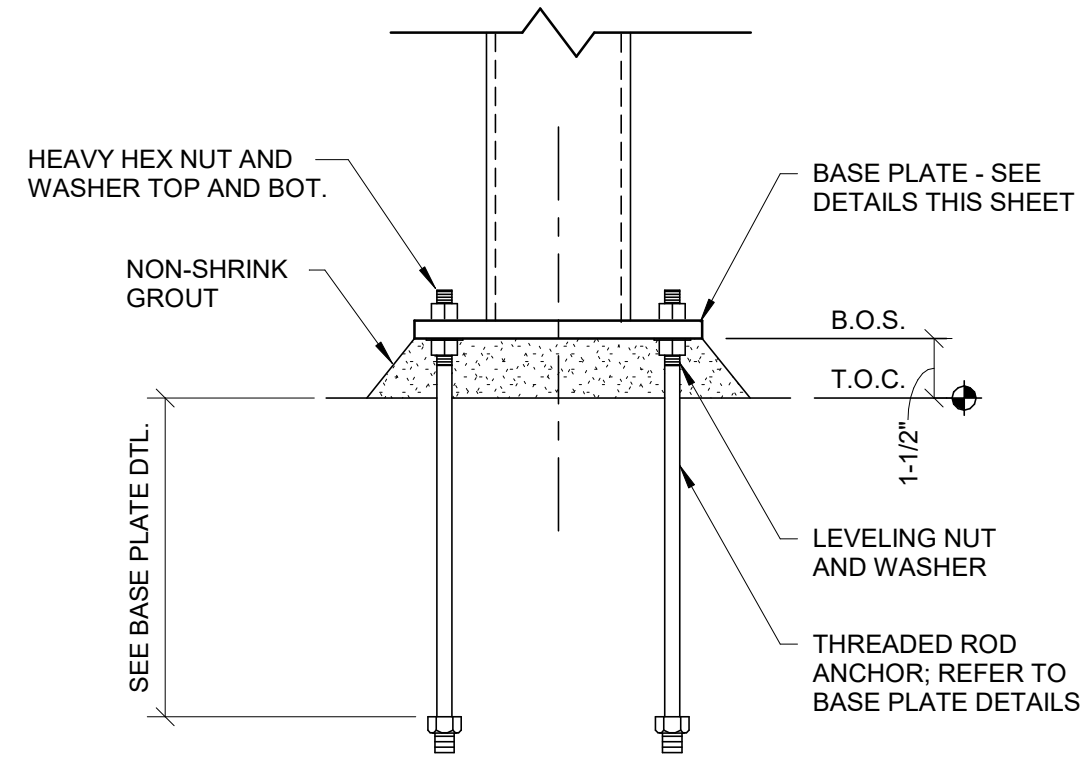
SITE DETAILS

S304

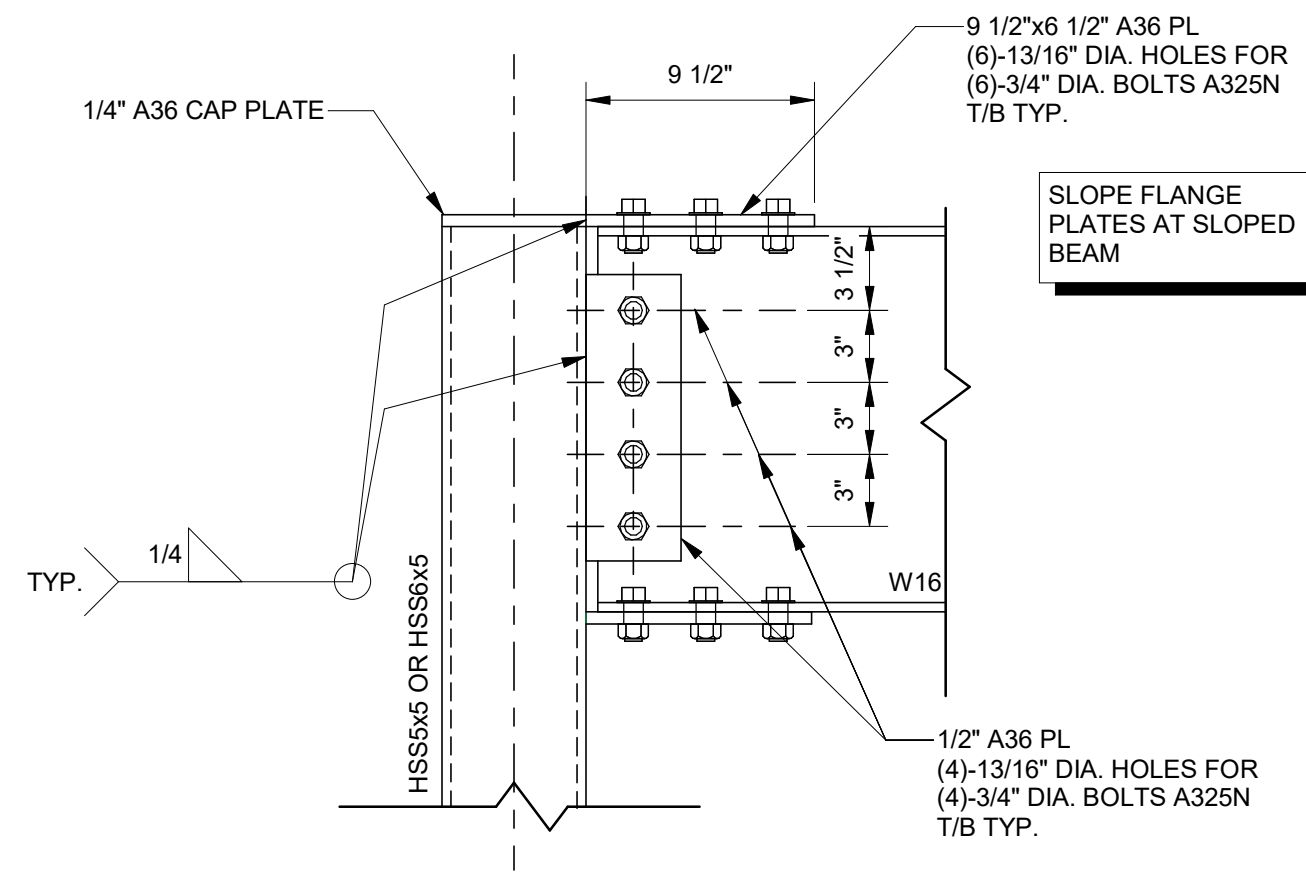




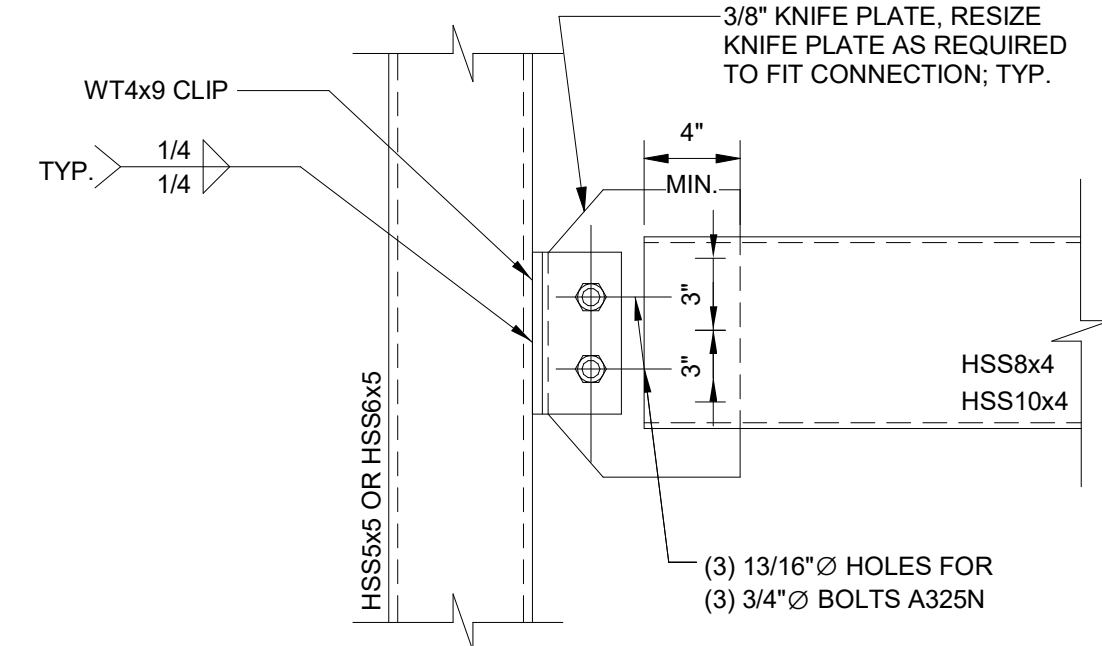
1 HSS5x5 BASE PLATE DETAIL  
Scale: 1 1/2" = 1'-0"



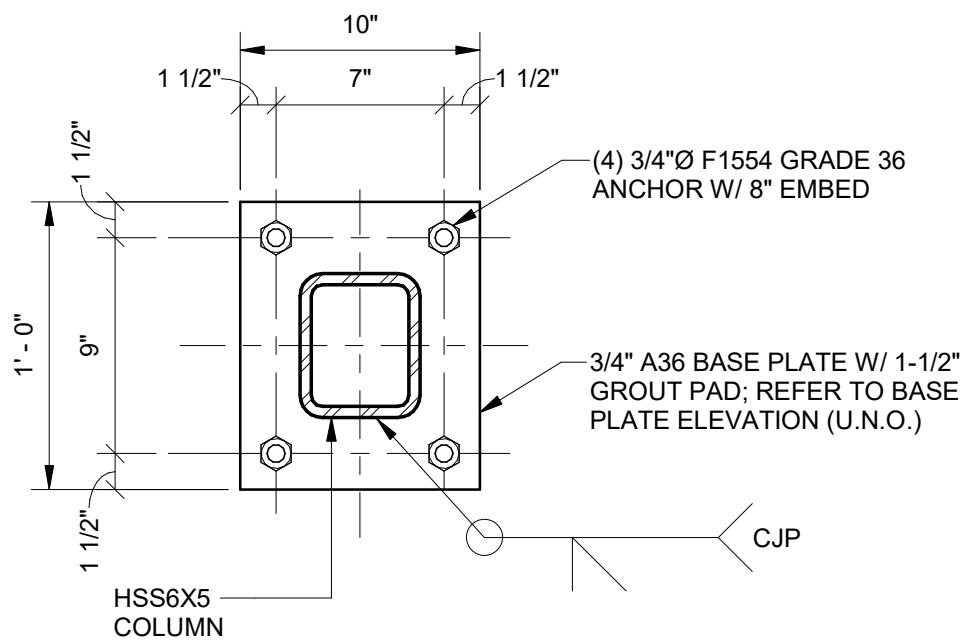
2 BASE PLATE W/ GROUT PAD ELEVATION  
Scale: 1 1/2" = 1'-0"



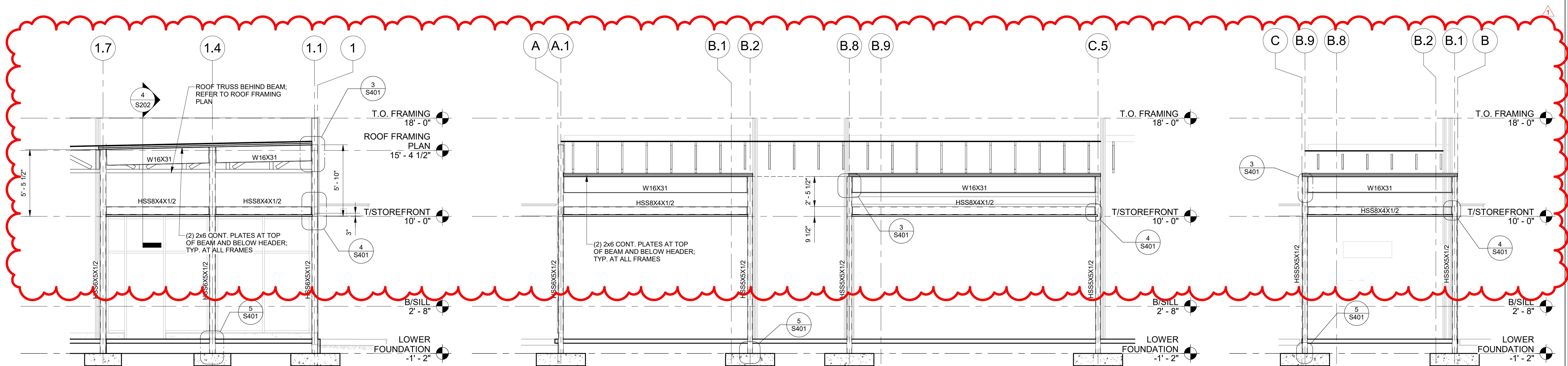
3 W16 TO HSS MOMENT CONNECTION DETAIL  
Scale: 1 1/2" = 1'-0"



4 HSS TO HSS SHEAR  
Scale: 1 1/2" = 1'-0"



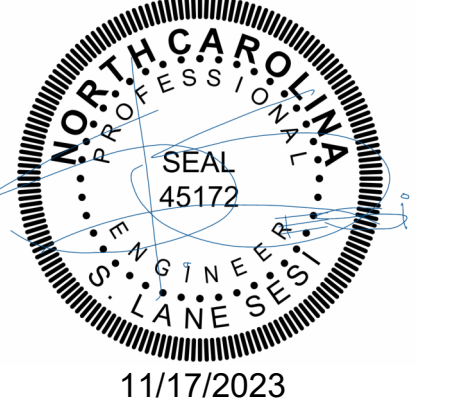
5 HSS6x5 BASE PLATE DETAIL  
Scale: 1 1/2" = 1'-0"



A Elevation 4 - a  
Scale: 1/4" = 1'-0"

B Elevation 5 - a  
Scale: 1/4" = 1'-0"

C Elevation 6 - a  
Scale: 1/4" = 1'-0"



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STEEL FRAMING  
DETAILS &  
ELEVATIONS

S401



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